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# **Topic 12.3 Concrete Pipe Culverts**

# 12.3.1

# Introduction

A common type of culvert used today is the concrete pipe culvert (see Figure 12.3.1). The concrete pipe culvert is typically circular or elliptical in shape. In situations where the required size of the opening is very large, two or more concrete pipe culverts may be used (see Figure 12.3.2).



Figure 12.3.1 Concrete Pipe Culvert

Culverts are somewhat protected by the soil backfill from rapid fluctuations in surface temperature and direct application chloride (salts) used for deicing. As a result they are generally more resistant to surface deterioration than concrete bridge elements.

See Topic P.3, Culverts, for a detailed presentation of various culvert types.



Figure 12.3.2 Twin Concrete Pipe Culvert

# 12.3.2

# Design Characteristics

# **Structural Behavior**

Concrete culverts are classified as rigid structures because they do not bend or deflect appreciably.

The load carrying capability of rigid culverts is essentially provided by the structural strength of the pipe itself and little benefit from the surrounding soil is required. When vertical loads are applied to rigid culverts, tension and compression zones are created (see Figure 12.3.3). Reinforcing steel is added to the tension zones to increase the tensile strength of the pipe. Shear stress in the haunch or "bell" area where the pipe sections are joined, can be critical for heavily loaded rigid pipe on hard foundations, especially if the pipe bed preparation is inadequate. Because rigid pipe is stiffer than the surrounding soil, it carries a substantial portion of the load.

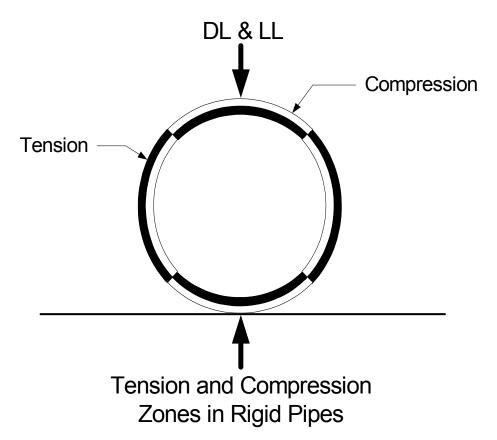


Figure 12.3.3 Rigid Culvert Stresses

The weight of earth that must be carried varies with soil characteristics and installation conditions. The installation conditions can have a significant influence on the loads that must be carried by a rigid culvert. There are two major classes of installation conditions: 1) trench, where culverts are placed in natural ground or compacted fill with a controlled trench width and 2) embankment, where culverts are placed in or covered by an embankment.

In narrow trench installations, the pipe is placed in a relatively narrow trench and covered with backfill material. The backfill tends to settle more than the undisturbed soil beside the trench. Friction between the backfill material and the sides of the trench tends to help support the backfill material reducing the load on the pipe. In effect the width of the soil column over the pipe is decreased.

As the trench width increases, the effect of the friction at the sides of the trench is reduced and dead load on the pipe is increased. The amount that the loading is increased depends on trench width and the amount of backfill settlement, which is related to compaction. Poorly compacted soil will settle more than well compacted soil. In a trench that is too wide, poor compaction can result in an increase in the dead load on the pipe. Pipes placed in a shallow bedding on top of the original ground surface and then covered by the embankment material will have loads similar to the very wide trench. Pipes placed in trenches in the original ground prior to being covered by embankment have reduced earth loads similar to those described for the narrow trench installations.

# 12.3.3

# Types and Shapes of Concrete Pipe Culverts

The size of the opening of the pipe is determined by the peak flow of the channel. The circular shape is the most common shape manufactured for pipe culverts. It is hydraulically and structurally efficient under most conditions. Possible hydraulic drawbacks are that circular pipe generally causes some reduction in stream width during low flows. It may also be more prone to clogging than some other shapes. Elliptical shapes are used in situations where horizontal or vertical clearance is limited. The oblong shape allows the pipe to fit where a circular pipe may not, but still allows for the necessary size opening. Elliptical shaped pipe culverts may also be used when a wider section is desirable for low flow levels. As with circular shaped pipe culverts, these shapes also are prone to clogging as the depth of flow increases.

Concrete culvert pipe is manufactured in up to five standard strength classifications. Higher classification numbers indicate higher strength. All of these standard shapes are manufactured in a wide range of sizes. Circular and elliptical pipes are available with standard sizes as large as 3.7 meters (12 feet) in diameter, with larger sizes available for special designs. Several factors such as span length, vertical and horizontal clearance, peak stream flow and terrain determine which shape of pipe culvert is used.

Precast concrete pipe culverts are manufactured in three standard shapes:

- Circular
- ➤ Horizontal elliptical
- Vertical elliptical

See pages 12.3.18 - 12.3.20 for the different standard sizes for concrete pipe culvert shapes.

# 12.3.4

# Hazards of Culvert Inspection

The bridge inspector should be alerted to the following hazards when inspecting a culvert.

- Inadequate ventilation
- Drowning
- > Toxic chemicals
- > Animals
- Quick conditions at the outlet

Refer to Topic 3.2.5 for a detailed discussion of each hazard.

# 12.3.5

# Overview of Common Defects

Common defects that occur on concrete pipe culverts include:

Cracking (flexure, shear, temperature, shrinkage, mass concrete)

SECTION 12: Special Bridges TOPIC 12.3: Concrete Pipe Culverts

- Scaling
- Delamination
- Spalling
- > Chloride contamination
- > Efflorescence
- **Ettringite formation**
- > Honeycombs
- Pop-outs
- Wear
- Collision damage
- Abrasion
- Overload damage
- Reinforcing steel corrosion
- Embankment erosion at culvert entrance and exit
- Roadway settlement
- Foundation Failure
- Scour / Undermining
- Misalignment
- > Settlement of pipe sections

Refer to Topic 2.2 for a detailed explanation of the properties of concrete, types and causes of concrete deterioration, and the examination of concrete.

# 12.3.6

# Inspection Procedures and Locations

Safety is the most important reason that culverts should be inspected. For a more detailed discussion on reasons for inspecting culverts, see Topic P.3.1.

Previous inspection reports and as-built plans, when available, should be reviewed prior to, and possibly during, the field inspection. A review of previous reports will familiarize the inspector with the structure and make detection of changed conditions easier. A review will also indicate critical areas that need special attention and the possible need for special equipment.

A logical sequence for inspecting culverts helps ensure that a thorough and complete inspection will be conducted. In addition to the culvert components, the inspector should also look for highwater marks, changes in the drainage area, settlement of the roadway, and other indications of potential problems. In this regard, the inspection of culverts is similar to the inspection of bridges.

For typical installations, it is usually convenient to begin the field inspection with general observations of the overall condition of the structure and inspection of the approach roadway. The inspector should select one end of the culvert and inspect the embankment, waterway, headwalls, wingwalls, and culvert barrel. The inspector should then move to the other end of the culvert. The following sequence is applicable to all culvert inspections:

- Review available information
- Observe overall condition

- > Inspect approach roadway and embankment
- Inspect waterway (see Topic 11.2)
- > Inspect end treatments
- ► Inspect culvert barrel

### **Procedures** Visual

The inspection of concrete pipe culverts for cracks, spalls, and other defects is primarily a visual activity.

### **Physical**

Sounding by a hammer can be used to detect delaminated areas. A delaminated area will have a distinctive hollow "clacking" sound when tapped with a hammer. A hammer hitting sound concrete will result in a solid "pinging" type sound.

# **Advanced Inspection Techniques**

Several advanced techniques are available for concrete inspection. Nondestructive methods, described in Topic 13.2.2, include:

- Acoustic wave sonic/ultrasonic velocity measurements
- > Delamination detection machinery
- **Electrical** methods
- **Electromagnetic methods**
- > Pulse velocity
- > Flat jack testing
- ➤ Ground-penetrating radar
- > Impact-echo testing
- > Infrared thermography
- Laser ultrasonic testing
- Magnetic field disturbance
- Neutron probe for detection of chlorides
- > Nuclear methods
- Pachometer
- Rebound and penetration methods
- ➤ Ultrasonic testing
- Radiography

Other methods, described in Topic 13.2.3, include:

- > Core sampling
- Carbonation
- Concrete permeability
- Concrete strength
- > Endoscopes and videoscopes
- ➤ Moisture content
- Petrographic examination

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- > Reinforcing steel strength
- Chloride test
- Matrix analysis
- > ASR evaluation

### Locations

Rigid culverts such as precast concrete pipe do not deflect appreciably before cracking or fracturing. As a result, shape inspections, while very important in flexible structures, are of little value in inspecting concrete culverts.

Although the need for soil stability and side support is obviously important with flexible pipe, it is less important with rigid pipe. However, adequate stability of the surrounding soil is necessary to prevent settlement around the culvert and to achieve load carrying capability. The inspector should therefore look for any indications of a lack of soil stability such as settlement or misalignment as well as signs of structural distress such as cracking. Descriptions of the types of distress to look for during inspection are provided in the following paragraphs. Guidelines for condition ratings of concrete pipe are included at the end of this Topic.

The following is a list of areas that should be inspected in concrete pipe culverts.

- Misalignment
- > Joint Defects
- > Cracks
- > Spalls
- Slabbing
- Durability
- ➤ End Section Drop-off

The following excerpts are from a reproduction of the out-of-print <u>Culvert Inspection Manual</u> Report No.-IP-86-2 – Chapter 5, Section 8.

# Misalignment

Misalignment may indicate the presence of serious problems in the supporting soil. The vertical and horizontal alignment of the culvert barrel should be checked by sighting along the crown and sides of the culvert and by checking for differential movement or settlement at joints between pipe sections. Vertical alignment should be checked for sags, faulting, and heaving. The inspector should be aware that pipes are occasionally laid with a camber or a grade change (broken back grade) to allow for fill settlement.

Sags which trap water may aggravate settlement problems by saturating the supporting soil. Horizontal alignment should be checked for straightness or smooth curvature for those culverts constructed with a curved alignment. Alignment problems may be caused by improper installation, undermining, or uneven settlement of fill. The inspector should attempt to determine which of those problems is causing the misalignment. If undermining is determined to be the probable cause, maintenance forces should be notified since damage will continue until the problem is corrected. The inspector should also try to determine whether the undermining is due to piping, water exfiltration, or infiltration of backfill

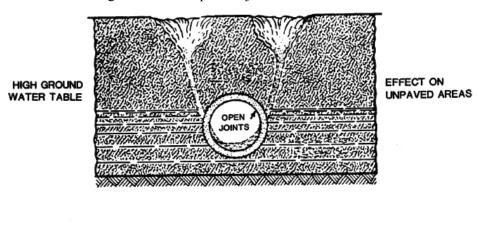
material. When the misalignment is due to improper installation or uneven settlement, repeat inspections may be needed to determine if the settlement is still progressing or has stabilized.

### Joint defects

Joint defects are fairly common and can range from minor problems to problems that are serious in nature. Typical joint defects include leakage (exfiltration and infiltration), cracks, and joint separation. Past and current criteria should be reviewed as some agencies design culverts with open joints to perform as subdrains.

- (1) Exfiltration Exfiltration occurs when leaking joints allow water flowing through the pipe to leak into the supporting material. Many culverts are built with joints that are not watertight or with mortar joints that crack with minor deflection, movement, or settlement of the pipe sections. Minor leakage may not always be a significant problem unless soils are quite erosive. However, if leaking joints contribute to or cause piping, then serious misalignment of the culvert or even failure may result. Leaking joints may be detected during low flows by visual observation of the joints and by checking around the ends of the culvert for evidence of piping.
- (2) Infiltration Infiltration is the opposite of exfiltration. Many culverts are essentially empty except during peak flows. When the water table is higher than the culvert invert, water may seep into the culvert between storms. This infiltration of water can cause settlement and misalignment problems if it carries fine grained soil particles from the surrounding backfill. Infiltration may be difficult to detect visually in its early stages although it may be indicated by open joints, staining at the joints on the sides and top of the culvert, deposits of soil in the invert, or by depressions over the culvert, as shown in Figure 12.3.4 (Exhibit 105).

(3) Cracks – Cracks in the joint area may be caused by improper handling during installation, improper gasket placement, and movement or settlement of the pipe sections. Cracked joints are more than likely not watertight even if gaskets were used. However, if no other problems are evident, such as differential movement between pipe sections, and the cracks are not open or spalling, they may be considered a minor problem to only be noted in the inspection report. Severe joint cracks are similar in significance to separated joints.



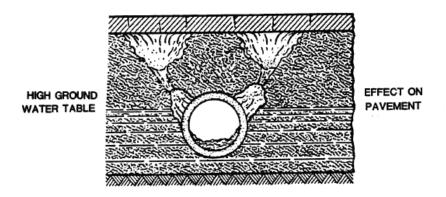
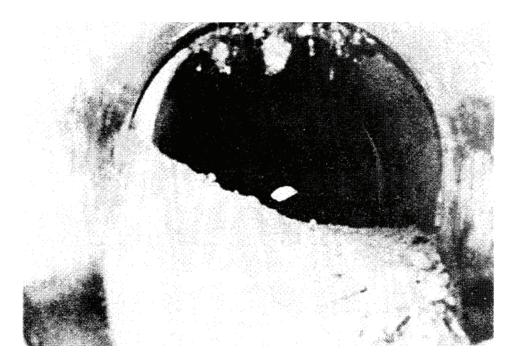
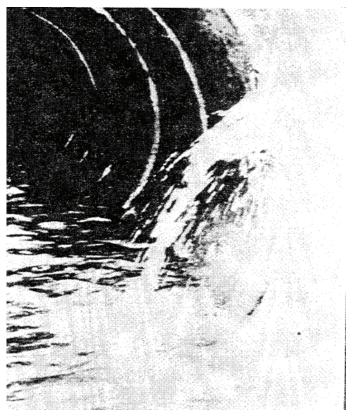


Figure 12.3.4 (Exhibit 105) Surface Indications of Infiltration

(4) Separated Joints - Joint separations may be caused by the same forces described under misalignment (settlement, undermining, or improper installation). Joint separations are significant because they accelerate damage caused by exfiltration and infiltration resulting in the erosion of the backfill material. Examples of severe infiltration through separate joints are shown in Exhibits 106 and 107. Separated joints are often found when severe misalignment is found. In fact either problem may cause or aggravate the other. Movement of the soil in the general direction of the culvert's centerline may cause sections to gradually pull apart. Embankment slippage may also cause separations to occur.



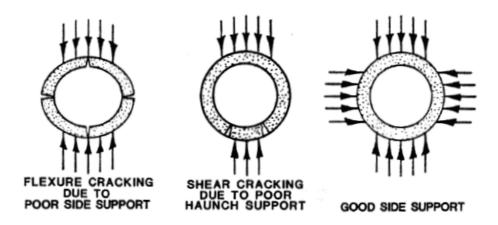
**Figure 12.3.5 (Exhibit 106)** Example of Severe Infiltration of Backfill Material through Separated Joints



**Figure 12.3.6 (Exhibit 107)** Severe Infiltration of Ground Water Through Separated Joint

### Cracks

Longitudinal Cracks – Concrete is strong in compression but weak in tension. Reinforcing steel is provided to handle the tensile stresses. Hairline longitudinal cracks in the crown or invert indicate that the steel has accepted part of the load. Cracks less than 0.01 inches in width are minor and only need to be noted in the inspection report. Cracks greater than hairline cracks, or those more than 0.01 inch in width but less than 0.1 inches, should be described in the inspection report and noted as possible candidates for maintenance. Longitudinal cracking in excess of 0.1 inch in width may indicate overloading or poor bedding. If the pipe is placed on hard material and backfill is not adequately compacted around the pipe or under the haunches of the pipe, loads will be concentrated along the bottom of the pipe and may result in flexure or shear cracking, as illustrated in Exhibit 108.



**Figure 12.3.7 (Exhibit 108)** Results of Poor and Good Side Support, Rigid Pipe

Other signs of distress such as differential movement, efflorescence, spalling, or rust stains should also be noted. Examples of longitudinal cracking are shown in Exhibits 109 and 110. When cracks are wider than 0.1 inch measurements should be taken of fill height and the diameter of the pipe both horizontally and vertically to permit analysis of the original design. Crack measurements and photographs may be useful for monitoring conditions during subsequent inspections.

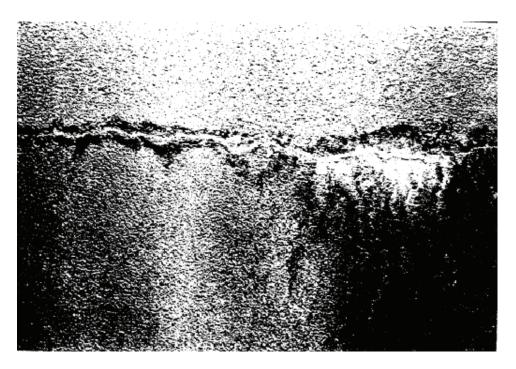
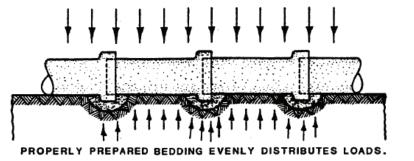


Figure 12.3.8 (Exhibit 109) Minor Longitudinal Crack with Efflorescence



**Figure 12.3.9 (Exhibit 110)** Severe Longitudinal Cracks with Differential Movement and Spalling

**Transverse Cracks** – Transverse or circumferential cracks may also be caused by poor bedding. Cracks can occur across the bottom of the pipe (broken bell) when the pipe is only supported at the ends of each section. This is generally the result of poor installation practices such as not providing indentions (bell holes) in hard foundation material for the ends of bell and spigot-type pipe or not providing a sufficient depth of suitable bedding material. Cracks may occur across the top of pipe (broken back) when settlement occurs and rocks or other areas of hard foundation material near the midpoint of a pipe section are not adequately covered with suitable bedding material. Transverse cracking is illustrated in Exhibit 111.



IMPROPERLY PREPARED BEDDING MAY RESULT IN STRESS CONCENTRATIONS.

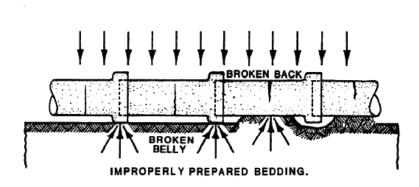


Figure 12.3.10 (Exhibit 111) Transverse or Circumferential Cracks

# **Spalls**

Spalling is a fracture of the concrete parallel or inclined to the surface of the concrete. In precast concrete pipe, spalls often occur along the edges of either longitudinal or transverse cracks when the crack is due to overloading or poor support rather than simple tension cracking. Spalling may also be caused by the corrosion of the steel reinforcing when water is able to reach the steel through cracks or shallow cover. As the steel corrodes, the oxidized steel expands, causing the concrete covering the steel to spall. Spalling may be detected by visual examination of the concrete along the edges of cracks. Tapping with a hammer should be performed along cracks to check for areas that have fractured but are not visibly separated. Such areas will produce a hollow sound when tapped. These areas may be referred to as delaminations or incipient spalls. Exhibit 112 shows spalling with reinforcing steel exposed.

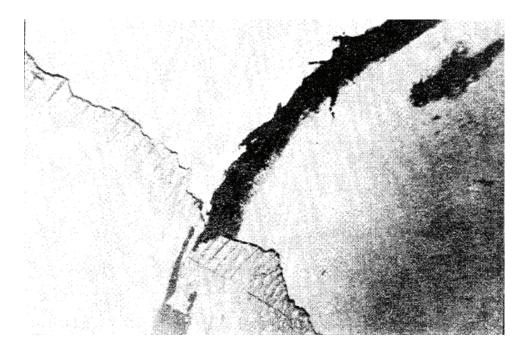


Figure 12.3.11 (Exhibit 112) Spalling Exposing Reinforcing Steel

# **Slabbing**

The terms slabbing, shear slabbing, or slab shear refer to a radial failure of the concrete which occurs from straightening of the reinforcement cage due to excessive deflection. It is characterized by large slabs of concrete "peeling" away from the sides of the pipe and a straightening of the reinforcing steel as shown in Exhibit 113. Slabbing is a serious problem that may occur under high fills.

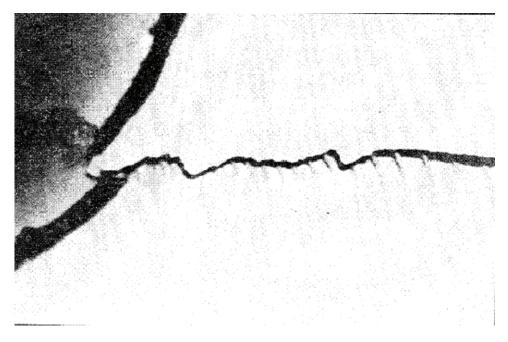


Figure 12.3.12 (Exhibit 113) Shear Slabbing

# **Durability**

Durability is a measure of a culvert's ability to withstand chemical attack and abrasion. Concrete pipes are subject to chemical attack in strongly acidic environments such as drainage from mines and may also be damaged by abrasion. Abrasion damage is a wearing away of the concrete surface by sediment and debris being transported by the stream. Mild deterioration or abrasion less than 1/4 inch deep should be noted in the report. More severe surface deterioration should be reported as a potential candidate for maintenance. In severe cases where the invert is completely deteriorated, maintenance forces should be given immediate notification. When linings are used to protect against chemical attack or abrasion the condition of the lining should be noted in the report.

# **End Section Drop-off**

This type of distress is usually due to outlet erosion as discussed earlier in the sections on end treatments and waterways. It is caused by the erosion of the material supporting the pipe sections on the outlet end of the culvert barrel.

As a starting point, select the lowest rating which matches actual conditions.

See Coding Guide for description of Rating Scale.

RATING	RATING GUIDELINES FOR PRECAST CONCRETE PIPE CULVERT BARRELS	LVERT BAR	IRELS
RATING	CONDITION	RATING	CONDITION
	book and a second		
6	New condition		
2 - 3 2   0   2   2   3   3   1	Alignment: good, no settlement or misalignment     Joints: tight with no defects apparent     Concrete: no cracking, spalling, or scaling present; surface in good condition	• car 2 **/	<ul> <li>Alignment: marginal; significant settlement and misalignment of pipe; evidence of piping; end sections dislocated about to drop off</li> <li>Joints: differential movement and separation of joints.</li> </ul>
ta nasori oli peliko Ald to Vi sed ga	<ul> <li>Alignment: generally good; minor misalignment at joints; no settlement</li> <li>Joints: minor openings, possible infiltration/exfiltration</li> <li>Concrete: minor hairline cracking at isolated locations; slight spalling or scaling present on invert</li> </ul>	e Latel By	significant infiltration or exfiltration at joints  • Concrete: cracks open more than 0.12 in. with efflorescence and spalling at numerous locations; spalls have exposed rebars which are heavily corroded; extensive surface scaling on invert greater than 0.5 in.  • Alignment: poor with significant ponding of water due to
one Tall Loga and Aus 2 Mart - J 10 G anno 1	Alignment: fair, minor misalignment and settlement at isolated locations     Joints: minor backfill infiltration due to slight opening at joints; minor cracking or spalling at joints allowing extiltration.      Concrete: extensive hairline cracks, some with minor delaminations or spalling; invert scaling less than 0.25 in.	JÖL BYBYTRO	sagging or misalignment pipes; end section drop off has occurred a Joints: significant openings, dislocated Joints in several locations exposing fill material; infiltration or exfiltration causing misalignment of pipe and settlement or depressions in roadway  • Concrete: extensive cracking, spalling, and minor slabbing; invert scaling has exposed reinforcing steel
() () () () () () () () () () () () () (	deep or small spalls present  Alignment: generally fair; minor misalignment or settlement throughout pipe; possible piping  Joints: open and allowing backfill to infiltrate; significant cracking or joint spalling  Concrete: cracking open greater than 0.12 in. with moderate demination and moderate spalling exposing reinforcing steel at isolated locations; large areas of invert with surface scaling or spalls greater than 0.25 in. deep	2 - 0 1	Alignment: critical; culvert not functioning due to alignment problems throughout  Problems throughout  Concrete: severe slabbing has occurred in culvert wall, invert concrete completely deteriorated in isolated locations  Culvert: partially collapsed  Road: closed to traffic  Culvert: total failure of culvert and fill  Road: closed to traffic
		ar sanci	

Figure 12.3.13 (Exhibit 114)

**Condition Rating Guidelines** 

**SECTION 12: Special Bridges** TOPIC 12.3: Concrete Pipe Culverts

Wingwalls and Headwalls Wingwalls are provided to support the embankment around the openings of the culvert. Wingwalls should be inspected to ensure they are in proper vertical alignment. Wingwalls may be tilted due to settlement, slides or scour. See Topic 10.1 for a detailed description of defects and inspection procedures of wingwalls.

# 12.3.7

# **Evaluation**

State and federal rating guideline systems have been developed to aid in the inspection of concrete pipe culverts. The two major rating guideline systems currently in use are the FHWA's Recording and Coding Guide for the Structural Inventory and Appraisal of the Nation's Bridges used for the National Bridge Inventory (NBI) component rating method and the AASHTO element level condition state assessment method.

# **NBI Rating Guidelines**

Using NBI rating guidelines, a 1-digit code on the Federal Structure Inventory and Appraisal (SI&A) sheet indicates the condition of the culvert (Item 62). This item evaluates the alignment, settlement, joints, structural condition, scour, and other items associated with culverts. Rating codes range from 9 to 0 where 9 is the best rating possible. See Topic 4.2 (Item 62) for additional details about NBI Rating Guidelines. The rating code is intended to be an overall evaluation of the culvert. Integral wingwalls to the first construction or expansion joint shall be included in the evaluation. It is also important to note that Items 58-Deck, 59-Superstructure, and 60-Substructure shall be coded "N" for all culverts.

The previous inspection data should be considered along with current inspection findings to determine the correct rating.

# **State Assessment**

Element Level Condition In an element level condition state assessment of a concrete pipe culvert, the AASHTO CoRe element is:

Element No.	<u>Description</u>
241	Reinforced Concrete Culvert

The unit quantity for culverts is meters or feet of culvert length along the barrel. The total quantity equals the culvert length times the number of barrels. The inspector must visually evaluate each 1 m (1 ft) slice of the culvert barrel(s) and assign the appropriate condition state description. The total length must be distributed among the four available condition states depending on the extent and severity of deterioration. The sum of the individual condition state quantities must equal the total element quantity. Condition state 1 is the best possible rating. See the AASHTO Guide for Commonly Recognized (CoRe) Structural Elements for condition state descriptions.

A Smart Flag is used when a specific condition exists, which is not described in the CoRe element condition state. The severity of the damage is captured by coding the appropriate Smart Flag condition state. The Smart Flag quantities are measured as each, with only one each of any given Smart Flag per bridge.

For settlement of the culvert, the "Settlement" Smart Flag, Element No. 360, can be used and one of three condition states assigned. For channel scour at the culvert ends, the "Scour" Smart Flag, Element No. 361, can be used and one of three condition states assigned.

# **Dimensions and Approximate Weights of Concrete Pipe**

\*ASTM C 76 – Reinforced concrete Culvert, Storm Drain and Sewer Pipe, Tongue and Groove Joints

	WALL A		WAl	LL B WALL C		
Internal Diameter inches	Minimum Wall Thickness, inches	Approximate Weight, pounds per foot	Minimum Wall Thickness, inches	Approximate Weight, pounds per foot	Minimum Wall Thickness, inches	Approximate Weight, pounds per foot
96	8	2710	9	3090	9 3/4	3355
102	8 1/2	3078	9 1/2	3480	10 1/4	3760
108	9	3446	10	3865	10 3/4	4160

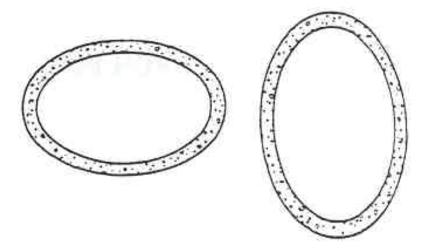
Large S	Large Sizes of Pipe Tongue and Groove Joint						
Internal Diameter Inches	Internal Diameter Feet	Wall Thickness Inches	Approximate Weight, pounds per foot				
114	9 ½	9 ½	3840				
120	10	10	4263				
126	10 ½	10 ½	4690				
132	11	11	5148				
138	11 ½	11 ½	5627				
144	12	12	6126				
150	12 ½	12 ½	6647				
156	13	13	7190				
162	13 ½	13 ½	7754				
168	14	14	8339				
174	14 ½	14 ½	8942				
180	15	15	9572				

<sup>\*</sup> For description of ASTM C 76 see page 12.3.20

Figure 12.3.14 Standard Sized for Concrete Pipe (Source: American Concrete Pipe Association)

# **Typical Cross Section of Arch Pipe**

Horizontal and Vertical Ellipse Pipe



# **Dimensions and Approximate Weights of Elliptical Concrete Pipe**

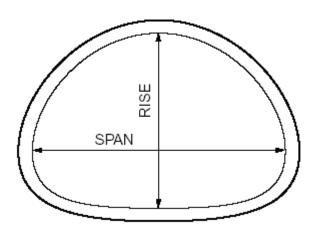
\*ASTM C 507 – Reinforced Concrete Elliptical Culvert, Storm Drain and Sewer Pipe

Equivalent Round Size, inches	Minor Axis, inches	Major Axis, inches	Minimum Wall Thickness, inches	Water-Way Area, square feet	Approximate Weight, pounds per foot
96	77	121	9 ½	52.4	3420
102	82	128	9 3/4	59.2	3725
108	87	136	10	66.4	4050
114	92	143	10 ½	74.0	4470
120	97	151	11	82.0	4930
132	106	166	12	99.2	5900
144	116	180	13	118.6	7000

Figure 12.3.14 Standard Sized for Concrete Pipe (Source: American Concrete Pipe Association), continued

<sup>\*</sup> For description of ASTM C 507 see page 12.3.20

**Typical Cross Section of Arch Pipe** 



# **Dimensions and Approximate Weights of Concrete Arch Pipe**

*ASTM C 506 – Reinforced Concrete Arch Culvert, Storm Drain and Sewer Pipe					
Equivalent Round Size, inches	Minimum Rise, inches	Minimum Span, inches	Minimum Wall Thickness, inches	Water-Way Area, square feet	Approximate Weight, pounds per foot
96	77 1/4	122	9	51.7	3110
108	87 1/8	138	10	66.0	3850
120	96 7/8	154	11	81.8	5040
132	106 1/2	168 3/4	10	99.1	5220

Figure 12.3.14 Standard Sized for Concrete Pipe (Source: American Concrete Pipe Association), continued

<sup>\*</sup> For description of ASTM C 506 see page 12.3.20

SECTION 12: Special Bridges TOPIC 12.3: Concrete Pipe Culverts

American Society for Testing and Materials (ASTM) descriptions for select rigid pipe culverts

- ASTM C 76 Reinforced concrete Culvert, Storm Drain, and Sewer Pipe: Covers reinforced concrete pipe intended to be used for the conveyance of sewage, industrial wastes, and storm waters, and for the construction of culverts. Class I 60 inches through 144 inches in diameter; Class II, III, IV and V 12 inches through 144 inches in diameter. Larger sizes and higher classes are available as special designs.
- ASTM C 506 Reinforced Concrete Arch Culvert, Storm Drain, and Sewer Pipe: Covers pipe to be used for the conveyance of sewage, industrial waste, and storm water and for the construction of culverts in sizes from 15 inch through 132 inch equivalent circular diameter. Larger sizes are available as special designs.
- ASTM C 507 Reinforced Concrete Elliptical Culvert, Storm Drain, and Sewer Pipe: Covers reinforced elliptically shaped concrete pipe to be used for the conveyance of sewage, industrial waste and storm water, and for the construction of culverts. Five standard classes of horizontal elliptical, 18 inches through 144 inches in equivalent circular diameter and five standard classes of vertical elliptical, 36 inches through 144 inches in equivalent circular diameter are included. Larger sizes are available as special designs.

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SECTION 12: Special Bridges TOPIC 12.4: Flexible Culverts

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# **Topic 12.4 Flexible Culverts**

# 12.4.1

# Introduction

Like all culverts, flexible culverts are designed for full flow. Unlike bridges, culverts have no distinction between substructure and superstructure and because earth backfill separates the culvert structure from the riding surface, culverts have no "deck." Most flexible culverts have a circular or elliptical configuration (see Figure 12.4.1). There are some flexible box and arch culverts in use today. From their design nature, flexible culverts have little structural bending strength without proper backfill. The material from which they are made, such as corrugated steel or aluminum can be flexed or bent and can be distorted significantly without cracking. Consequently, flexible culverts depend on the backfill support to resist bending. In flexible culvert designs, proper interaction between the soil and structure is critical.



Figure 12.4.1 Pipe Arch Flexible Culvert

# 12.4.2

# Design Characteristics

# **Structural Behavior**

A flexible culvert is a composite structure made up of the culvert barrel and the surrounding soil. The barrel and the soil are both vital elements to the structural performance of the culvert.

Flexible pipe has relatively little bending stiffness or bending strength on its own. Flexible culvert materials include steel, aluminum, and plastic. As loads are applied to the culvert, it attempts to deflect. In the case of a round pipe, the vertical diameter decreases and the horizontal diameter increases (see Figure 12.4.2). When good embankment material is well compacted around the culvert, the increase in horizontal diameter of the culvert is resisted by the lateral soil pressure. With round pipe the result is a relatively uniform radial pressure around the pipe which creates a compressive thrust in the pipe walls. As illustrated in Figure 12.4.3, the compressive thrust is approximately equal to vertical pressure times one-half the span length.

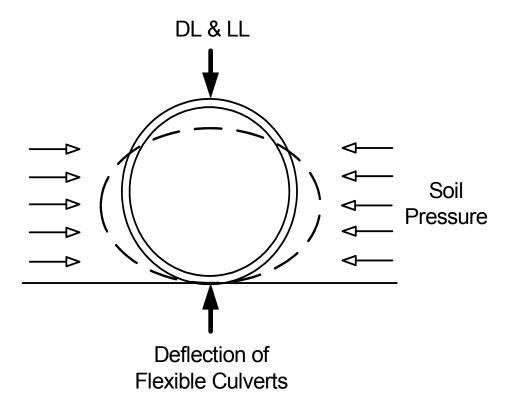


Figure 12.4.2 Flexible Culvert Deflection

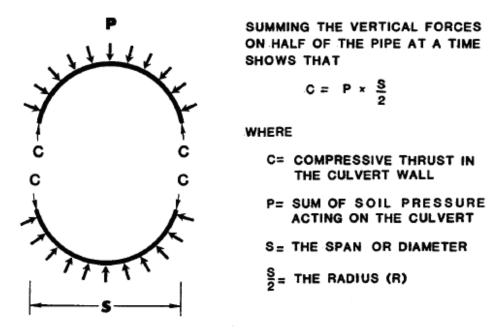


Figure 12.4.3 Formula for Ring Compression

An arc of a flexible round pipe, or other shape will be stable as long as adequate soil pressures are achieved, and as long as the soil pressure is resisted by the compressive force C on each end of the arc. Good quality backfill material and proper installation are critical in obtaining a stable soil envelope around a flexible culvert.

In long span culverts the radius (R) is usually large. To prevent excessive deflection due to dead and/or live loads, longitudinal or circumferential stiffeners are sometimes added. The circumferential stiffeners are usually metal ribs bolted to the outside of the culvert. Longitudinal stiffeners may be metal or reinforced concrete. Concrete thrust beams provide some circumferential stiffening as well as longitudinal stiffening. The thrust beams are added to the structure prior to backfill. They also provide a solid vertical surface for soil pressures to act on and a surface which is easier to backfill against. The use of concrete stress relieving slabs is another method used to achieve longer spans or reduce minimum cover. A stress-relieving slab is cast over the top of the backfill above the structure to distribute live loads to the adjacent soil.

# 12.4.3

# **Types and Shapes** of Flexible Culverts

Flexible culverts are constructed from corrugated steel or aluminum pipe or field assembled structural plate products. Structural plate steel products are available as structural plate pipes, box culverts, or long span structures. See Figure 12.4.4 for standard shapes for corrugated flexible culverts.

	Shape	Range of Sizes	Common Uses
Round	0	6 in – 26 ft	Culverts, subdrains, sewers, service tunnels, etc. All plates same radius. For medium and high fills (or trenches).
Vertically- elongated (ellipse) 5% is common	0	4-21 ft nominal: before elongating	Culverts, sewers, service tunnels, re- covery tunnels. Plates of varying radii; shop fabrication. For appearance and where backfill compaction is only moderate.
Pipe-arch	Rise	Span x Rise 18 in. x 11 in. to 20 ft 7 in. x 13 ft 2 in.	Where headroom is limited. Has hydraulic advantages at low flows. Corner plate radius, 18 inches or 31 inches for structural plate.
Underpass*	Rise	Span x Rise 5 ft 8 in. x 5 ft 9 in. to 20 ft 4 in. x 17 ft 9 in.	For pedestrians, livestock or vehicles (structural plate).
Arch	Rise	Span x Rise 6 ft x 1 ft 9½ in. to 25 ft x 12 ft 6 in.	For low clearance large waterway open- ing, and aesthetics (structural plate).
Horizontal Ellipse	Span	Span 20-40 ft	Culverts, grade separations, storm sewers, tunnels.
Pear	Span	Span 25–30 ft	Grade separations, culverts, storm sewers, tunnels.
High Profile Arch	Span	Span 20-45 ft	Culverts, grade separations, storm sewers, tunnels, Ammo ammunition mag azines, earth covered storage.
Low Profile Arch	Soan	Span 20-50 ft	Low-Wide waterway enclosures, culverts storm sewers.
Box Culverts	Span	Span 10-21 ft	Low-wide waterway enclosures, culverts storm sewers.
	Specials	Various	For lining old structures or othe special purposes. Special fabrication

<sup>\*</sup>For equal area or clearance, the round shape is generally more economical and simpler to assemble.

**Figure 12.4.4 (Exhibit 11 Culvert Inspection Manual Report No. FHWA-IP-86-2)** Standard Corrugated Steel Culvert Shapes (Source: Handbook of Steel Drainage and Highway Construction Products, American Iron and Steel Institute)

# **Corrugated Pipe**

Factory-made pipe is produced in two basic shapes: round and pipe arch. Both shapes are produced in several wall thicknesses, several corrugation sizes, and with annular (circumferential) or helical (spiral) corrugations. Pipes with helical corrugations have continuously welded seams or lock seams. Both round and arch steel pipe shapes are available in a wide range of standard sizes. Round pipe is available in standard sizes up to 3.7 m (12 feet) in diameter. Standard sizes for pipe arch are available in sizes up to the equivalent of a 3 m (10 feet) diameter round pipe. Round aluminum pipe is available in standard sizes up to 3 m (10 feet) in nominal diameter. Aluminum pipe arch is available in sizes up to the equivalent of an 2.4 m (8 feet) diameter round pipe.

Flexible aluminum culverts are constructed from factory assembled corrugated aluminum pipe or field assembled from structural plates. Structural plate aluminum culverts are available as conventional structural plate structures, box culverts, or long span structures.

### **Structural Plate**

Structural plate steel pipes are field assembled from standard corrugated galvanized steel plates. Standard plates have corrugations with a 150 mm (6-inch) pitch and a depth of 50 mm (2 inches). Plates are manufactured in a variety of thicknesses and are pre-curved for the size and shape of structure to be erected.

Structural steel plate pipes are available in four basic shapes:

- Round
- Pipe arch
- > Arch
- Underpass

The standard sizes available range in span from 1.5 to 7.9 m (5 feet to 26 feet).

Structural plate aluminum pipes are field assembled with a 230 mm (9-inch) pitch and a depth of 65 mm (2.5 inches).

Structural plate aluminum pipes are produced in five basic shapes:

- Round
- Pipe arch
- > Arch
- Pedestrian/animal underpass
- Vehicle underpass

A wide range of standard sizes is available for each shape. Spans as large as 9.1 m (30 feet) can be obtained for the arch shape.

### **Box Culverts**

Corrugated steel box sections use standard corrugated galvanized steel plates with special reinforcing elements applied to the areas of maximum moments. Steel box culverts are available with spans that range from to 2.9 to 6.3 m (9 feet 8 inches to 20 feet 9 inches).

The aluminum box culvert utilizes standard aluminum structural plates with

aluminum rib reinforcing added in the areas of maximum bending stresses. Ribs are bolted to the exterior of the aluminum shell during installation. Aluminum box culverts are suitable for shallow depths of fill and are available with spans ranging from to 2.7 to 7.7 m (8 feet 9 inches to 25 feet 5 inches).

# **Long Span Culverts**

Long span steel culverts are assembled using conventional 150 by 50 mm (6 by 2 inch) corrugated galvanized steel plates and longitudinal and circumferential stiffening members. There are five standard shapes for long span steel structures:

- ➤ Horizontal elliptical
- Pipe arch
- ➤ Low profile arch
- ➤ High profile arch
- Pear shape

The span lengths of typical sections range from 5.9 to 12.2 m (19 feet 4 inches to 40 feet). Longer spans are available for some shapes as special designs. It should be noted that each long span installation represents, to a certain extent, a custom design. The inspector should therefore use design or as-built plans when checking dimensions of existing long span structures.

Long span aluminum structures are assembled using conventional 230 by 65 mm (9 by 2 1/2 inch) corrugated aluminum plates and aluminum rib stiffeners. Long span aluminum structures are essentially the same size and available in the same five basic shapes as steel long spans.

See the end of this Topic for the different standard sizes for each flexible culvert shape (pg 164-193 Culvert Inspection Manual Report No. FHWA-IP-86-2)

# 12.4.4

# Hazards of Culvert Inspection

The bridge inspector should be alert to the following hazards when inspecting a culvert.

- > Inadequate ventilation
- Drowning
- > Toxic chemicals
- Animals
- Quick conditions at the outlet

Refer to Topic 3.2.5 for a detailed discussion of each hazard.

### 12.4.5

# Overview of Common Defects

Common defects that can occur to flexible culvert materials include the following:

- Pitting
- Surface Rust

- Section Loss
- Overload Damage
- ➤ Heat Damage
- Buckling
- Embankment erosion at culvert entrance and exit
- Roadway settlement

Refer to Topic 2.3 for a more detailed presentation of the properties of steel, types and causes of steel deterioration, and the examination of steel.

# 12.4.6

# Inspection Procedures and Locations

Safety is an important reason that culverts should be inspected. For a more detailed discussion on reasons for inspecting culverts, see Topic P.3.1.

A logical sequence for inspecting culverts helps ensure that a thorough and complete inspection will be conducted. In addition to the culvert components, the inspector should also look for highwater marks, changes in the drainage area, settlement of the roadway, and other indications of potential problems. In this regard, the inspection of culverts is similar to the inspection of bridges.

For typical installations, it is usually convenient to begin the field inspection with general observations of the overall condition of the structure and inspection of the approach roadway. The inspector should select one end of the culvert and inspect the embankment, waterway, headwalls, wingwalls, and culvert barrel. The inspector should then move to the other end of the culvert. The following sequence is applicable to all culvert inspections:

- Review available information
- Observe overall condition
- Inspect approach roadway and embankment
- Inspect waterway (see in Topic 11.2)
- > Inspect end treatments
- ➤ Inspect culvert barrel

# **Procedures**

# Visual

Most defects in flexible culverts are first detected by visual inspection. In order for this to occur, a hands-on inspection, or inspection where the inspector is close enough to touch the area being inspected, is required. The types of defects to look for when inspecting the culvert barrel will depend upon the type of culvert being inspected. In general, corrugated metal culvert barrels should be inspected for cross-sectional shape and barrel defects such as joint defects, seam defects, plate buckling, lateral shifting, missing or loose bolts, corrosion, excessive abrasion, material defects, and localized construction damage. A critical area for the inspection of long span metal culverts is at the 2 o'clock and 10 o'clock locations. An inward bulge at these locations may indicate potential failure of the structure.

# Physical

In a steel culvert, the bolts on the longitudinal seams should be checked by tapping

the nuts with a hammer. For aluminum structural plate, the bolts should be checked with a torque wrench.

A geologist's pick hammer can be used to scrape off heavy deposits of rust and scale. The hammer can then be used to locate areas of corrosion by striking the culvert walls. The walls will deform or the hammer will break through the culvert wall if severe corrosion exists.

Sometimes surveying the culvert is necessary to determine if there is any shape distortion, and if there is distortion how much exists.

# **Advanced Inspection Techniques**

In metal culverts, visual inspections can only point out surface defects. Therefore, advanced inspection techniques may be used to achieve a more rigorous and thorough inspection of the flexible culvert, including:

Several advanced techniques are available for steel inspection. Nondestructive methods, described in Topic 13.3.2, include:

- Acoustic emissions testing
- Computer programs
- Computer tomography
- Corrosion sensors
- Smart paint 1
- > Smart paint 2
- Dye penetrant
- Magnetic particle
- Radiographic testing
- Robotic inspection
- Ultrasonic testing
- **Eddy current**

Other methods, described in Topic 13.3.3, include:

- Brinell hardness test
- > Charpy impact test
- Chemical analysis
- > Tensile strength test

# **Locations Inspect End Treatments**

End treatments should be inspected like any other structural component. Their effectiveness can directly affect the performance of the culvert.

The most common types of end treatments for flexible culverts are:

- Projections
- Mitered
- Pipe end section

**Projections** - The inspector should indicate the location and extent of any scour or undermining around the culvert ends. The depth of any scouring should be measured with a probing rod. In low flow conditions scour holes have a tendency to fill up with debris or sediment. If no probing rod is used an inspector could mistakenly report less scour than has taken place.

Water flowing along the outside of a culvert can remove supporting material. This is referred to as piping and it can lead to the culvert end being unsupported. If not repaired in time, piping can cause cantilevered end portions of the culvert to bend down and restrict the stream flow.

**Mitered Ends** - Inspection items for mitered ends are the same as for projecting ends. Additional care should be taken to measure any deformation of the end. Mitering the end of corrugated pipe culvert reduces its structural capacity.

**Pipe End Sections** - These are typically used on relatively smaller culverts. For inspection purposes, treat the pipe end section as you would a projected end.

The following excerpts are from a reproduction of the out-of-print <u>Culvert Inspection Manual</u> Report No.-IP-86-2 – Chapter 5, Section 4.

### Section 4 - CORRUGATED METAL CULVERTS

#### 5-4.0 General

Corrugated aluminum and corrugated steel culverts are classified as flexible structures because they respond to and depend upon the soil backfill to provide structural stability and support to the culvert. The flexible corrugated metal acts essentially as a liner. The liner acts mainly in compression and can carry large ring compression thrust, but very little bending or moment force. (Rib reinforced box culverts are exceptions.) Inspection of the culvert determines whether the soil envelope provides adequate structural stability for the culvert and verifies that the "liner" is capable of carrying the compressive forces and protecting the soil backfill from water flowing through the culvert. Verification of the stability of the soil envelope is accomplished by checking culvert shape. Verification of the integrity of the "liner" is accomplished by checking for pipe and plate culvert barrel defects.

This section contains discussions on inspecting corrugated metal structures for shape and barrel defects. Because shape inspection requirements do vary somewhat for different shapes, separate sections with detailed guidelines are provided for corrugated metal pipe culvert shapes and long-span culvert shapes. Section 5 of this chapter addresses corrugated metal pipe culverts, and section 6 covers long-span corrugated metal culverts.

# 5-4.1 Shape Inspections

The single most important feature to observe and measure when inspecting corrugated metal culverts is the cross-sectional shape of the culvert barrel. The corrugated metal culvert barrel depends on the backfill or embankment to maintain its proper shape and stability. When the backfill does not provide the required support, the culvert will deflect, settle, or distort. Shape changes in the culvert therefore provide a direct indication of the adequacy and stability of the supporting soil envelope. By periodic observation and measurement of the culvert's shape, it is possible to verify the adequacy of the backfill. The design or theoretical cross-section of the culvert should be the standard against which field measurements and visual observations are compared. If the design cross section is unknown, a comparison can be made between the unloaded culvert ends and the loaded sections beneath the roadway or deep fills. This can often provide an indication of structure deflection or settlement. Symmetrical shape and uniform curvature around the perimeter are generally the critical factors. If the curvature around the structure becomes too flat, and/or the soil continues to yield under load, the culvert wall may not be able to carry the ring thrust without either buckling inward or deflecting excessively to the point of reverse curvature. Either of these events leads to partial or total failure.

As explained in earlier in this Topic, an arc of a circular pipe or other shape structure will be stable and perform as long as the soil pressure on the outside of the pipe is resisted by the compression force in the pipe at each end of the arc.

Corrugated metal pipes can change shape safely within reasonable limits as long as there is adequate exterior soil pressure to balance the ring compression. Therefore, size and shape measurements taken at any one time do not provide conclusive data on backfill instability even when there is significant deviation from the design shape. Current backfill stability cannot be reliably determined unless changes in shape are measured over time. It is therefore necessary to identify current or recent shape changes to reliably check backfill stability. If there is instability of the backfill, the pipe will continue to change shape.

In general, the inspection process for checking shape will include visual observations for symmetrical shape and uniform curvature as well as measurements of important dimensions. The specific measurements to be obtained depend upon factors such as the size, shape, and condition of the structure. If shape changes are observed, more measurements may be necessary. For small structures in good condition, one or two simple measurements may be sufficient, for example, measuring the horizontal diameter on round pipe. For larger structures such as long span culverts, key measurements may be difficult to obtain. Horizontal diameters may be both high and large. The inspection process for long span culverts generally requires that elevations be established for key points on the structure. Although some direct measurements may also be required for long-span structures, elevations are needed to check for settlement and for calculating vertical distances such as the middle ordinate of the top arc. For structures with shallow cover, observations of the culvert with a few live loads passing over are recommended. Discernible movement in the structure may indicate possible instability and a need for more in-depth investigation.

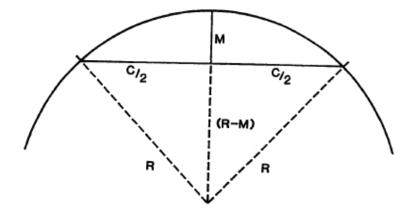
The number of measurement locations depends upon the size and condition of the

structure. Long-span culverts should normally be measured at the end and at 7.6 m (25 foot) intervals. Measurements may be required at more frequent intervals if significant shape changes are observed. The smaller pipe culverts can usually be measured at longer intervals than long-span culverts.

Locations in sectional pipe can be referenced by using pipe joints as stations to establish the stationing of specific cross-sections. Stations should start with number 1 at the outlet and increase going upstream to the inlet. The location of points on a circular cross section can be referenced like hours on a clock. The clock should be oriented looking upstream. On structural plate corrugated metal culverts, points can be referenced to bolted circumferential and longitudinal seams.

It is extremely important to tie down exact locations of measurement points. Unless the same point is checked on each inspection, changes cannot be accurately monitored. The inspection report must, therefore, include precise descriptions of reference point locations. It is safest to use the joints, seams, and plates as the reference grid for measurement points. Exact point locations can then be easily described in the report as well as physically marked on the structures. This guards against loss of paint or scribe marks and makes points easy to find or reestablish. All dimensions in structures should be measured to the inside crest of corrugation. When possible, measurement points on structural plate should be located at the center of a longitudinal seam. However, some measurement points are not on a seam.

When distortion or curve flattening is apparent, the extent of the flattened area, in terms of arc length, length of culvert affected, and the location of the flattened area should be described in the inspection report. The length of the chord across the flattened area and the middle ordinate of the chord should be measured and recorded. The chord and middle ordinate measurements can be used to calculate the curvature of the flattened area using the formula shown in Exhibit 66.



C = MEASURED CHORD

M = MEASURED MIDDLE ORDINATE

SOLVE FOR RA = ACTUAL RADIUS

$$R_A = \frac{4M^2 + C^2}{8M}$$

IF R<sub>A</sub> IS > R<sub>D</sub> (DESIGN RADIUS) THEN ACTUAL CURVE IS FLATTER THAN DESIGN

Figure 12.4.5 (Exhibit 66) Checking Curvature by Curve and Middle Ordinate

### 5-4.2 Inspecting Barrel Defects

The structural integrity of corrugated metal culverts and long-span structures is dependent upon their ability to perform in ring compression and their interaction with the surrounding soil envelope. Defects in the culvert barrel itself, which can influence the culvert's structural and hydraulic performance, are discussed in the following paragraphs. Rating guidelines are provided in the sections dealing with specific shapes.

a. Misalignment - The inspector should check the vertical and horizontal alignment of the culvert. The vertical alignment should be checked visually for sags and deflection at joints. Poor vertical alignment may indicate problems with the subgrade beneath the pipe bedding. Sags trap debris and sediment and may impede flow. Since most highway culverts do not have watertight joints, sags which pocket water could saturate the soil beneath and around the culvert, reducing the soil's stability. The horizontal alignment should be checked by sighting along the sides for straightness. Vertical alignment can be checked by sighting along bolt lines. Minor horizontal and vertical misalignment is generally not a significant problem in corrugated metal structures unless it causes shape or joint problems. Occasionally culverts are intentionally

installed with a change in gradient.

b. Joint Defects - Field joints are generally only found with factory manufactured pipe. There are ordinarily no joints in structural plate culverts, only seams. (In a few cases, preassembled lengths of structural plate pipe have been coupled or banded together like factory pipe.)

Field joints in factory pipe serve to maintain the water conveyance of the culvert from section to section, to keep the pipe sections in alignment, keep the backfill soil from infiltrating, and to help prevent sections from pulling apart. Joint separation may indicate a lack of slope stability as described in section 5-4.2 e., circumferential seams. Key factors to look for in the inspection of joints are indications of backfill infiltration and water exfiltration. Excessive seepage through an open joint can cause soil infiltration or erosion of the surrounding backfill material reducing lateral support. Open joints may be probed with a small rod or flat rule to check for voids. Indications of joint defects include open joints, deflection, seepage at the joints, and surface sinkholes over the culvert as illustrated in Exhibits 67 and 68. Any evidence of joint defects should be recorded. Culverts in good condition should have no open joints, those in fair condition may have a few open joints but no evidence of soil infiltration, and those in marginal to poor condition will show evidence of soil infiltration.

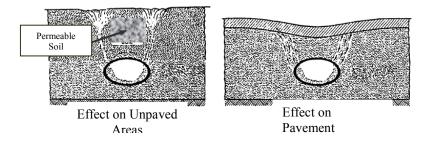


Figure 12.4.6 (Exhibit 67) Surface Indications of Infiltration



Figure 12.4.7 (Exhibit 68) Surface Hole Above Open Joint

c. Seam Defects in Fabricated Pipe - Pipe seams in helical pipe do not carry a significant amount of the ring compression thrust in the pipe. That is the reason that a lock seam is an acceptable seam. Helical seams should be inspected for cracking and separation. An open seam could result in a loss of backfill into the pipe, or exfiltration of water. Either condition could reduce the stability of the surrounding soil.

In riveted or spot welded pipes, the seams are longitudinal and carry the full ring compression in the pipe. These seams, then, must be sound and capable of handling high compression forces. They should be inspected for the same types of defects as those described in the text for structural plate culverts, Section 12.4.3, Structural Pipe. When inspecting the longitudinal seams of bituminous-coated corrugated metal culverts, cracking in the bituminous coating may indicate seam separation.

d. Longitudinal Seam Defects in Structural Plate Culverts - Longitudinal seams should be visually inspected for open seams, cracking at bolt holes, plate distortion around the bolts, bolt tipping, cocked seams, cusped seams, and for significant metal loss in the fasteners due to corrosion.

Culverts in good condition should have only minor joint defects. Those in fair condition may have minor cracking at a few bolt holes or minor opening at seams that could lead to infiltration or exfiltration. Marginal to poor culvert barrel conditions are indicated by significant cracking at bolt holes, or deflection of the structure due to infiltration of backfill through an open seam. Cracks 3 inches (76 mm) long on each side of the bolts indicate very poor to critical conditions.

(1) Loose Fasteners - Seams should be checked for loose or missing fasteners as shown in Exhibit 69. For steel structures the

longitudinal seams are bolted together with high-strength bolts in two rows; one row in the crests and one row in the valleys of the corrugations. These are bearing type connections and are not dependent on a minimum clamping force of bolt tension to develop interface friction between the plates. Fasteners in steel structural plate may be checked for tightness by tapping lightly with a hammer and checking for movement.



**Figure 12.4.8 (Exhibit 69)** Close-Up of Loose and Missing Bolts at a Cusped Seam; Loose Fasteners are Usually Detected by Tapping the Nuts with a Hammer

For aluminum structural plate, the longitudinal seams are bolted together with normal strength bolts in two rows with bolts in the crests and valleys of both rows. These seams function as bearing connections, utilizing bearing of the bolts on the edges of holes and friction between the plates. The seams in aluminum structural plate should be checked with a torque wrench (125 ft-lbs (169 Joules) minimum to 150 ft-lbs (203 Joules) maximum). If a torque wrench is not available fasteners can be checked for tightness with a hammer as described for steel plates.

(2) Cocked and Cusped Seams - The longitudinal seams of structural plate are the principal difference from factory pipe. The shape and curvature of the structure is affected by the lapped, bolted longitudinal seam. Improper erection or fabrication can result in cocked seams or cusped effects in the structure at the seam, as illustrated in Exhibit 70. Slight cases of these conditions are fairly common and frequently not significant. However, severe cases can result in failure of the seam or structure. When a cusped seam is significant the structure's shape appearance and key dimensions will differ significantly from the design shape and dimensions.

The cusp effect should cause the structure to receive very low ratings on the shape inspection if it is a serious problem. A cocked seam can result in loss of backfill and may reduce the ultimate ring compression strength of the seam.

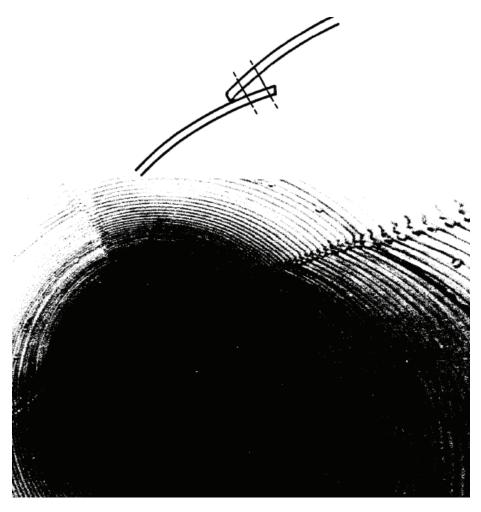


Figure 12.4.9 (Exhibit 70) Cocked Seam with Cusp Effect

(3) Seam Cracking - Cracking along the bolt holes of longitudinal seams can be serious if allowed to progress. As cracking progresses, the plate may be completely severed and the ring compression capability of the seam lost. This could result in deformation or possible failure of the structure. Longitudinal cracks are most serious when accompanied by significant deflection, distortion, and other conditions indicative of backfill or soil problems. Longitudinal cracks are caused by excessive bending strain, usually the result of deflection, Exhibit 71. Cracking may occasionally be caused by improper erection practices such as using bolting force to "lay down" a badly cocked seam.

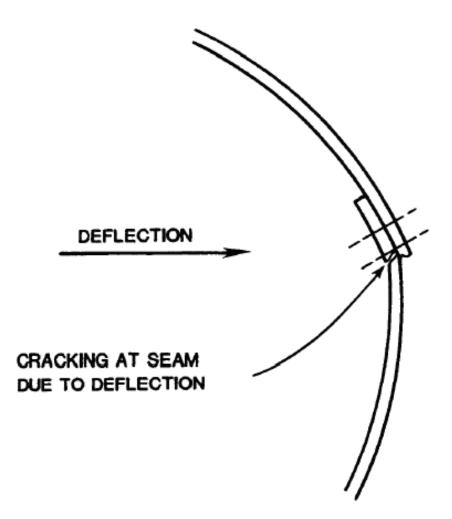
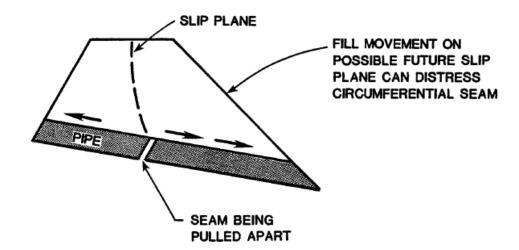


Figure 12.4.10 (Exhibit 71) Cracking Due to Deflection

- (4) Bolt Tipping The bolted seams in structural plate culverts only develop their ultimate strength under compression. Bolt tipping occurs when the plates slip. As the plates begin to slip, the bolts tip, and the bolt holes are plastically elongated by the bolt shank. High compressive stress is required to cause bolt tipping. Structures have rarely been designed with loads high enough to produce a ring compression that will cause bolt tip. However, seams should be examined for bolt tip particularly in structures under higher fills. Excessive compression on a seam could result in plate deformations around the tipped bolts and failure is reached when the bolts are eventually pulled through the plates.
- e. Circumferential Seams The circumferential seams, like joints in factory pipe, do not carry ring compression. They do make the conduit one continuous structure. Distress in these seams is rare and will ordinarily be a result of a severe differential deflection or distortion problem or some other manifestation of soil failure. For example, a steep sloping structure through an embankment may be pulled apart longitudinally if the embankment moves down as shown in Exhibit 72. Plates should be installed with the upstream plate overlapping the downstream plate to provide a "shingle" effect in the

direction of flow.



**Figure 12.4.11 (Exhibit 72)** Circumferential Seam Failure Due to Embankment Slippage

The circumferential seam at one or more locations would be distressed by the movement of the fill. Such distress is important to note during inspections since it would indicate a basic problem of stability in the fill. Circumferential seam distress can also be a result of foundation failure, but in such cases should be clearly evident by the vertical alignment.

- f. Dents and Localized Damage All corrugated metal culverts should be inspected for localized damage. Pipe wall damage such as dents, bulges, creases, cracks, and tears can be serious if the defects are extensive and can impair either the integrity of the barrel in ring compression or permit infiltration of backfill. Small, localized examples are not ordinarily critical. When the deformation type damages are critical, they will usually result in a poorly shaped cross section. The inspector should document the type, extent, and location of all significant wall damage defects. When examining dents in corrugated steel culverts, the opposite side of the plate should be checked, if possible, for cracking or disbonding of the protective coating.
- g. Durability (Wall Deterioration) Durability refers to the ability of a material to resist corrosion and abrasion. Corrosion is the deterioration of metal due to electrochemical or chemical reactions. Abrasion is the wearing away of culvert materials by the erosive action of bedload carried in the stream.

Abrasion is generally most serious in steep or mountainous areas where high flow rates carry sand and rocks that wear away the culvert invert. Abrasion can also accelerate corrosion by wearing away protective coatings.

Metal culverts are subject to corrosion in certain aggressive environments. For example, steel rapidly corrodes in salt water and in environments with highly acidic (low pH) conditions in the soil and water. Aluminum is fairly resistant to salt water but will corrode rapidly in highly alkaline (high pH) environments, particularly if metals such as iron or copper and their salts are present. The

electrical resistivity of soil and water also provide an indication of the likelihood of corrosion. Many agencies have established guidelines in terms of pH and resistivity that are based on local performance. The FHWA has also published guidelines for aluminum and steel culverts including various protective coatings.

Corrosion and abrasion of corrugated metal culverts can be a serious problem with adverse effects on structural performance. Damage due to corrosion and abrasion is the most common cause for culvert replacement. The inspection should include visual observations of metal corrosion and abrasion. As steel corrodes it expands considerably. Relatively shallow corrosion can produce thick deposits of scale. A geologist's pick-hammer can be used to scrape off heavy deposits of rust and scale permitting better observation of the metal. A hammer can also be used to locate unsound areas of exterior corrosion by striking the culvert wall with the pick end of the hammer. When severe corrosion is present, the pick will deform the wall or break through it. Protective coatings should be examined for abrasion damage, tearing, cracking, and removal. The inspector should document the extent and location of surface deterioration problems.

When heavy corrosion is found by observation or sounding, special inspection methods such as pH testing, electrical resistivity measurement, and obtaining cores from the pipe wall are recommended. A routine program for testing pH and electrical resistivity should be considered since it is relatively easy to perform and provides valuable information.

Durability problems are the most common cause for the replacement of pipe culverts. The condition of the metal in corrugated metal culverts and any coatings, if used, should be considered when assigning a rating to the culvert barrel. Suggested rating guidelines for metal culverts with metallic coatings are shown in Exhibit 73. Modification of these guidelines may be required when inspecting culverts with non-metallic coatings. Aluminum culvert barrels may be rated as being in good condition if there is superficial corrosion. Steel culverts rated as in good condition may have superficial rust with no pitting. Perforation of the invert as shown in Exhibit 74 would indicate poor condition. Complete deterioration of the invert in all or part of the culvert barrel would indicate a critical condition as shown in Exhibit 75. Culverts with deteriorated inverts may function as an arch structurally, but are highly susceptible to failure due to erosion of the bedding.

Rating <u>Value</u>	General Description	Corrugated Steel	Corrugated Aluminum
9	New	Near original condition	Near original condition
8	Good	Superficial rust, no pitting	Superficial corrosion slight pitting
7	Generally Good	Moderate rust, slight pitting	Moderate corrosion no attack of core alloy
6	Fair	Fairly heavy rust, moderate pitting, slight thinning	Significant corrosion minor attack of core alloy
5	Generally Fair	Extensive heavy rust, deep pitting, moderate thinning	Significant corrosion moderate attack of core alloy
4	Marginal	Pronounced thinning (some deflection or penetration when struck with pick hammer)	Extensive corrosion significant attack of core alloy
3	Poor	Extensive heavy rust, deep pitting scattered perforations	Extensive corrosion attack of core alloy scattered perforations
2	Critical	Extensive perforations due to rust	Extensive perforations due to corrosion
1	Critical	Invert completely deteriorated	Invert completely deteriorated
0	Critical	Partial or complete collapse	Partial or complete collapse

**Figure 12.4.12 (Exhibit 73)** Suggested Rating Criteria for Condition of Corrugated Metal

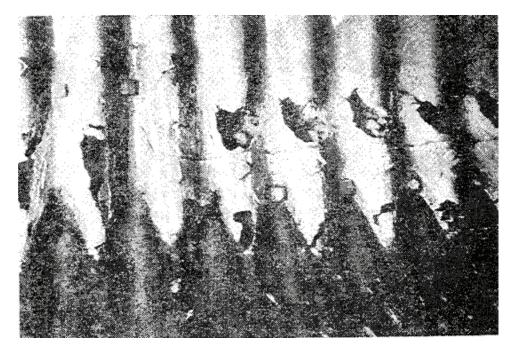


Figure 12.4.13 (Exhibit 74) Perforation of the Invert Due to Corrosion



Figure 12.4.14 (Exhibit 75) Invert Deterioration

h. Concrete Footing Defects - Structural plate arches, long-span arches, and box culverts use concrete footings. Metal footings are occasionally used for the arch and box culvert shapes. The metal "superstructure" is dependent upon the footing to transmit the vertical load into the foundation. The structural plate arch is usually bolted in a base channel which is secured in the footing.

The most probable structural defect in the footing is differential settlement. One section of a footing settling more than the rest of the footing can cause wrinkling or other distortion in the arch. Flexible corrugated metal culverts can tolerate some differential settlement but will be damaged by excessive differential settlement. Uniform settlement will not ordinarily affect a metal arch but can affect the clearances in a grade separation structure if the footings settle and the road does not. The significance of differential footing settlement increases as the amount of the difference in settlement increases, the length it is spread over decreases, and the height of the arch decreases. This concept is illustrated in Exhibit 76.

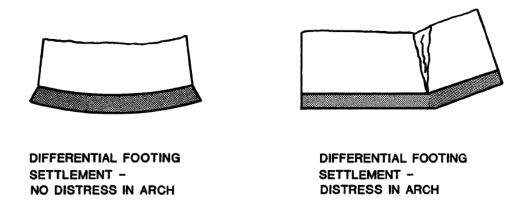


Figure 12.4.15 (Exhibit 76) Differential Footing Settlement

The inspection of footings in structural plate and long-span arches should include a check for differential settlement along the length of a footing. This might show up in severe cracking, spalling, or crushing across the footing at the critical spot. If severe enough, it might be evidenced by compression or stretching of the corrugations in the culvert barrel. Deterioration may occur in concrete and masonry footings which is not related to settlement but is caused by the concrete or mortar. In arches with no invert slab, the inspector should check for erosion and undermining of the footings and look for any indication of rotation of the footing as illustrated in Exhibits 77 and 78.

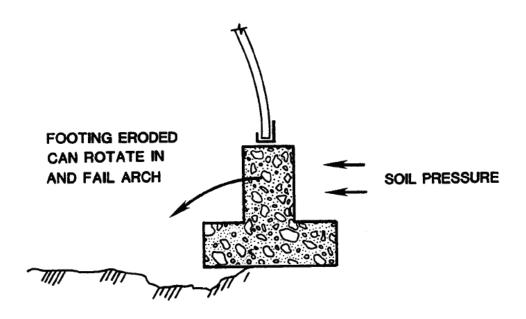


Figure 12.4.16 (Exhibit 77) Footing Rotation due to Undermining

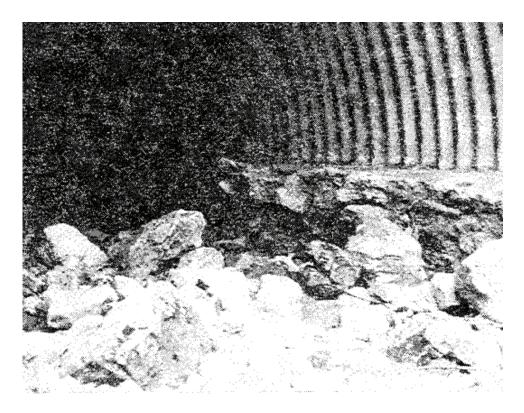


Figure 12.4.17 (Exhibit 78) Erosion of Invert Undermining footing of Arch

Culverts rated in good condition may have minor footing damage. Poor to critical condition would be indicated by severe footing undermining, damage, or rotation, or by differential settlement causing distortion and circumferential kinking in the corrugated metal as shown in Exhibit 79.



Figure 12.4.18 (Exhibit 79) Erosion Damage to Concrete Invert

i. Defects in Concrete Inverts - Concrete inverts in arches are usually floating slabs used to carry water or traffic. Invert slabs provide protection against erosion and undercutting, and are also used to improve hydraulic efficiency. Concrete inverts are sometimes used in circular, as well as other culvert shapes, to protect the metal from severe abrasive or severe corrosive action. Concrete invert slabs in arches should be checked for undermining and damage such as spalls, open cracks, and missing portions. The significance of damage will depend upon its effect on the footings and corrugated metal.

The following excerpts are from a reproduction of the out-of-print <u>Culvert Inspection Manual</u> (Supplement to Manual 70), July 1986 – Chapter 5, Section 5.

## Section 5 - SHAPE INSPECTION OF CORRUGATED METAL CULVERT BARRELS

### 5-5.0 General

This section deals with shape inspections of common culvert shapes including round and vertical elongated, pipe arches, arches, and box culvert shapes. Specific guidelines for recommended measurements to be taken for each location are provided for each typical culvert shape. Additional measurements are also recommended when field measurements differ from the design dimensions or when significant shape changes are observed. Rating guidelines are also provided for each shape. The guidelines include condition descriptions with shape and barrel defects defined for each rating.

### 5-5.1 Using the Rating Guidelines

When using the rating guidelines, the inspector should keep the following factors in mind:

- a. The inspector should select the lowest rating which best describes either the shape condition or the barrel condition. Structure shape is the most critical factor in flexible culverts, and this should be kept in mind when selecting the rating.
- b. The shape criteria described for each numerical rating should be considered as a group rather than as separate criteria for each measurement check listed. Good curvature and the rate of change are critical. Significant changes in shape since the last inspection should be carefully evaluated even if the structure is still in fairly good condition.
- c. The guidelines merely offer a starting point for the inspector. The inspector must still use judgment in assigning the appropriate numerical rating. The numerical rating should be related to the actions required. The inspector may wish to refer to Section 4.2 of this manual.

## 5-5.2 Round and Vertical Elongated Pipe

Round and vertically elongated pipes are expected to deflect vertically during construction resulting in a slightly increased horizontal span. Round pipes are sometimes vertically elongated five percent to compensate for settlement during construction. It is frequently difficult to determine in the field if a pipe was round or elongated when installed. Large round pipes may appear to be elongated if they were subjected to minor flattening of the sides during backfill.

Vehicular underpasses sometimes use 10 percent vertically elongated very large pipe which is susceptible to side flattening during installation. In shallow cover situations, adequate curvature in the sides is the important factor. The soil pressures on the sides may be greater than the weight of the shallow fill over the pipe. The result is a tendency to push the sides inward rather than outward as in deeper buried or round pipes. Side flattening, such as that shown in Exhibit 80, can be caused by unstable backfill. A deteriorated invert may have contributed to the problem by reducing the pipe's ability to transmit compressive forces.

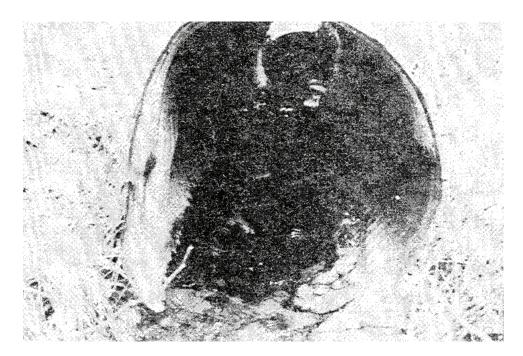
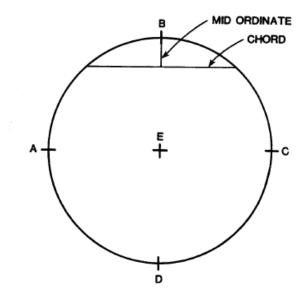


Figure 12.4.19 (Exhibit 80) Excessive Side Deflection

Flattening of the top arc is an indication of possible distress. Flattening of the invert is not as serious. Pipes not installed on shaped bedding will often exhibit minor flattening of the invert arc. However, severe flattening of the bottom arc would indicate possible distress.

The inspector should note the visual appearance of the culvert's shape and measure the horizontal span as shown in Exhibit 81. Almost all round or vertical elongated pipe can be directly measured and will not require elevations. Exceptions are large vertical elongated grade separation structures. On such structures, elevations should be obtained similar to those recommended for the long-span pear shape.



- 1. MINIMUM MEASUREMENTS REQUIRED:
  - HORIZONTAL DIAMETER = AC
- 2. IF FLATTENING OBSERVED MEASURE:
  - CHORD AND MID ORDINATE OF FLATTENED AREA
- 3. IF HORIZONTAL DIAMETER EXCEEDS DESIGN BY MORE THAN 10% MEASURE:
  - VERTICAL DIAMETER = BD

**Figure 12.4.20 (Exhibit 81)** Shape Inspection Circular and Vertical Elongated Pipe

If the visual appearance or measured horizontal diameter differ significantly from the design specifications, additional measurement, such as vertical diameter, should be taken. Flattened areas should be checked by measuring a chord and the mid ordinate of the chord. The chord length and ordinate measurement should be noted in the report with a description of the location and extent of the flattened area.

Round and vertically elongated pipe with good to fair shape will have a generally good shape appearance. Good shape appearance means that the culvert's shape appears to match the design shape, with smooth, symmetrical curvature and no visible deformations. The horizontal span should be within 10 percent of the design span. Pipe with marginal shape will be indicated by characteristics such as a fair or marginal general shape appearance, distortion in the upper half of the pipe, severe flattening in the lower half of the pipe, or horizontal spans 10 to 15 percent greater than design.

Pipe with poor to critical shape will have a poor shape appearance that does not match the design shape, does not have smooth or symmetrical curvature, and may have obvious deformations. Severe distortion in the upper half of the pipe, a

horizontal diameter more than 15 percent to 20 percent greater than the design diameter, or flattening of the crown to an arc with a radius of 20 to 30 feet or more would indicate poor to critical condition. It should be noted that pipes with deflection of less than 15 to 20 percent may be rated as critical based on poor shape appearance. Guidelines for rating round corrugated metal culvert are presented in Exhibit 82.

starting point, select the lowest rating which matches actual conditions.

See Coding Guide for description of Rating Scale.

RATING	CONDITION	RATING	NOILIUNOO
	. 40 01 01		
6	New condition		
8	• Shape: good, smooth curvature in barrel	4	<ul> <li><u>Shape</u>: marginal significant distortion throughout length of pipe, lower third may be kinked</li> </ul>
	<ul> <li>Horizontal: within 10 percent of design</li> <li>Seans and Joints: tight, no openings</li> </ul>		- Horizontal Diameter: 10 percent to 15 percent greater than design
	Metal:     Aluminum: superficial corrosion, slight pitting		• Seams or Joints: Moderate cracking at bolt holes on one seam
			near tup or pipe, deflection caused by loss of backfill through open joints
	Shape: generally good, top half of pipe smooth but minor flattening of bottom		- Alunium: extensive corrosion, significant attack of core
	r: within 10 per		- Steel: extensive heavy rust, deep pitting
	o Seams or Joints: minor cracking at a few bolt holes, minor joint or seam openings, potential for backfill infiltration	٠	10
2	• Metal:	,	flattening of crown crown radius 20 to 20 cost
18	- Aluminum: moderate corrosion, no attack of core alloy - Steel: moderate rust, slight pitting		- Horizontal Diameter: in excess if 15 percent greater than
21		.33	• Seans: 3 in. long cracks at bolt holes on one seam
•	<ul> <li><u>Shabe</u>: fair, top half has smooth curvature but bottom half has flattened significantly</li> </ul>	124	Metal:
	- Horizontal Diameter: within 10 percent of design		- Aluminum: extensive corrosion, attack of core alloy, scattered perforations
	esam in lower half of sing Caidons of handit incitation	-34	- Steel: extensive heavy rust, deep pitting, scattered
	gh seams or joints		
	• Metal:	2	· Shape: critical, extreme distortion and deflection throughout
004	- Aluminum: Significant corrosion, minor attack of core alloy - Steel: fairly heavy rust, moderate pitting		pipe, flattening of crown, crown radius over 30 feet - Horizontal Diameter: More than 20 percent greater than design
s	• Shape: generally fair, significant distortion at isolated		<ul> <li>Seams: plate cracked from bolt to bolt on one seam</li> <li>Metal:</li> </ul>
31	locations in top half and extreme flattening of invert		- Aluminum: extensive perforations due to corrosion
	- norizontal Diameter: 10 percent to 15 percent greater than design		- Steel: extensive perforations due to rust
0		- A	• Shape: partially collapsed with crown in reverse curve
	near bottom of pipe, deflection of pipe caused by backfill infiltration through seams or ininte		• Seams: failed
10	• Hetal:		WOAD: Closed to traffic
	- Aluminum: significant corrosion, moderate attack of core	0	• Pipe: totally failed
15%	- Steel: scattered heavy rust, deep pitting		

Figure 12.4.21 (Exhibit 82) Condition Rating Guidelines

## 5-5.3 Pipe Arch

The pipe arch is a completely closed structure but is essentially an arch. The load is transmitted to the foundation principally at the corners. The corners are much like footings of an arch. There is relatively little force or pressure on the large radius bottom plate. The principal type of distress in a pipe arch is a result of inadequate soil support at the corners where the pressure is relatively high. The corner may push down or out into the sail while the bottom stays in place. The effect will appear as if the bottom pushed up. This problem is illustrated in Exhibits 83 and 84.

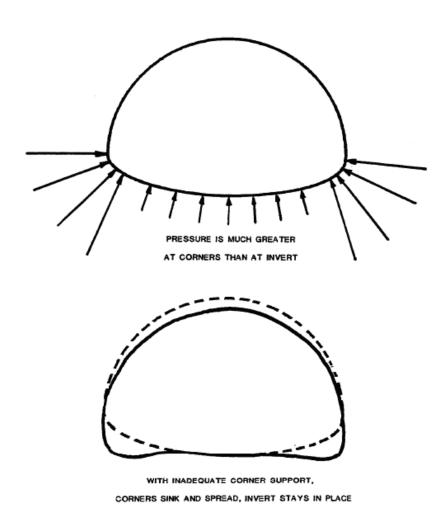


Figure 12.4.22 (Exhibit 83) Bottom Distortion in Pipe Arches

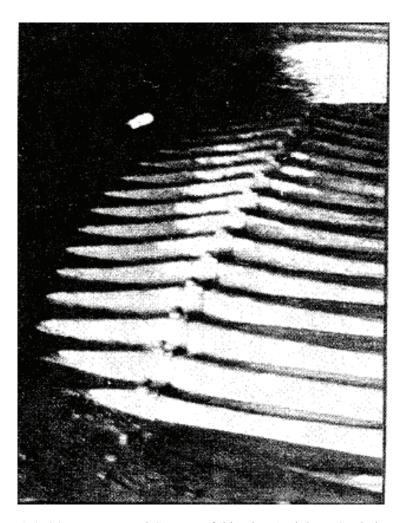
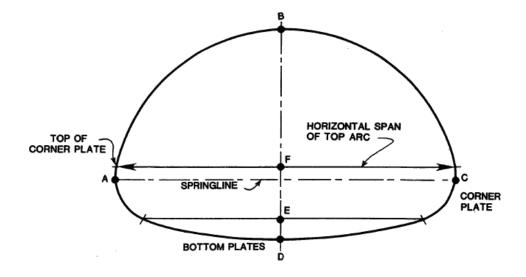


Figure 12.4.23 (Exhibit 84) Bottom and Corners of this Pipe Arch have Settled

The bottom arc should be inspected for signs of flattening and the bottom corners for signs of spreading. The extent and location of bottom flattening and corner spreading should be noted in the inspection report.

Complete reversal of the bottom arc can occur without failure if corner movement into the foundation has stabilized. The top arc of the structure is supporting the load above and its curvature is an important factor. However, if the "footing" corner should fail, the top arc would also fail. The spreading of the corners is therefore very important as it affects the curvature of the top arc.

The inspector should record the visual appearance of the shape and measure both the span and the rise. If the span exceeds the design span by more than 3 percent, the span of the top arc, the mid ordinate of the top arc, and the mid ordinate of the bottom arc should also be measured. Recommended measurements are shown in Exhibit 85.



- 1. MINIMUM REQUIRED MEASUREMENTS AC. BD
  - SPAN = AC
  - RISE = BD

2. IF AC EXCEEDS DESIGN BY 3% OR MORE MEASURE BF, ED, AND HORIZONTAL SPAN OF TOP ARC

Figure 12.4.24 (Exhibit 85) Shape Inspection Structural Plate Pipe Arch

Pipe arches in fair to good condition will have a symmetrical appearance, smooth curvature in the top of the pipe, and a span less than five percent greater than theoretical. The bottom may be flattened but should still have curvature. Pipe arches in marginal condition will have fair to marginal shape appearance, with distortion in the top half of the pipe, slight reverse curvature in the bottom of the pipe, and a horizontal span five to seven percent greater than theoretical. Pipe in poor to critical condition will have characteristics such as a poor shape appearance, severe deflection or distortion in the top half of the pipe, severe reverse curvature in the bottom of the pipe, flattening of one side, flattening of the crown to an arc with a radius of 6.1 to 9.1 m (20 to 30 feet), or a horizontal span more than seven percent greater than theoretical. Guidelines for rating pipe arches are shown in Exhibit 86.

As a starting point, select the lowest rating which matches actual conditions.

See Coding Guide for description of Rating Scale.

RATING	RATING GUIDELINES FOR CORRUGATED METAL PIPE-ARCH BARRELS	H BARRELS	
RATING	CONDITION	RATING	CONDITION
e v v v v v v v v v v v v v v v v v v v	• New condition • Shape: good with smooth curvature - Horizontal Span: less than 3 percent greater than design • Joints or Seame: good condition • Aluminum: superficial curvoin, slight pitting - Steel: superficial rust, no pitting - Stape: generally good, smooth curvature in top half, bottom flattened but still curved - Horizontal Span: within 3 to 5 percent greater than design Joints or Seame; minor cracking at a few bolt holes; minor Joints or Seame; minor cracking at a few bolt holes; minor - Aluminum: moderate corrosion, no attack of core alloy - Steel: moderate corrosion, no attack of core alloy - Steel: moderate rust, slight pitting - Joints or Seame: minor cracking all along one seam; minor joint openings with evidence of infiltration - Joints or Seame: minor cracking all along one seam; minor joint openings with evidence of infiltration - Steel: fairly heavy rust, moderate pitting in one location; bottom has slight reverse curvature in one location - Steel: fairly heavy rust, moderate pitting in one location; bottom has slight reverse curvature in one location - Horizontal Span: within 5 to 7 percent greater than design - Joints and Seame: moderate cracking at boit holes along a seam in one section, backfill being lost through seam or joint causing slight deflection - Hetal: - Aluminum: significant corrosion, moderate attack of core alloy - Steel: scattered heavy rust, deep pitting	(3½, (3), 1, 16; 1, 1), 1; 4 8 2 1	• <u>Shape</u> : marginal, significant distortion all along top of arch, bottom has reverse curve  - ilorizontal Span: more than 7 percent greater than design backfill infiltration causing major deflection  - Muninum: extensive corrosion, significant attack of core allorise poor, extreme deflection in top arch in one section; bottom has reverse curvature throughout  - Steel: extensive heavy rust, deep pitting  - Shape: poor, extreme deflection in top arch in one section; bottom has reverse curvature throughout  - incrizontal Span: more than 7 percent greater than design scattered perforations  - Steel: extensive heavy rust, deep pitting, scattered perforations  - Steel: extensive heavy rust, deep pitting, scattered perforations  - Steel: extensive perforations due to corrosion  - Muninum: extensive perforations due to corrosion  - Steel: extensive perforations due to rust  - Shape: seam failed  - Shape: structure partially collapsed  - Shape: structure collapsed

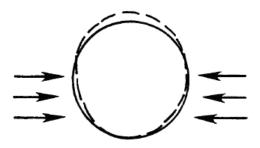
Figure 12.4.25 (Exhibit 86) Condition Rating Guidelines

#### **5-5.4** Arches.

Arches are fixed on concrete footings, usually below or at the springline. The springline is a line connecting the outermost points on the sides of a culvert. This difference between pipes and arches means that an arch tends to deflect differently during backfill. Backfill forces tend to flatten the arch sides and peak its top because the springline cannot move inward like the wall of a round pipe as shown in Exhibit 87. As a result, important shape factors to look for in an arch are flattened sides, peaked crown, and flattened top arc.



# BACKFILL TENDS TO PEAK ARCHES (DOTTED LINE)



## ROUND PIPES CAN DEFLECT MORE UNIFORMLY

Figure 12.4.26 (Exhibit 87) Arch Deflection During Installation

Another important shape factor in arches is symmetrical shape. If the arch was erected with the base channels not square to the centerline, it causes a racking of the cross section. A racked cross-section is one that is not symmetrical about the centerline of the culvert. One side tends to flatten, the other side tends to curve more while the crown moves laterally and possibly upward. If these distortions are not corrected before backfilling the arch, they usually get worse during backfill. Exhibit 88 illustrates racked or peaked arches.

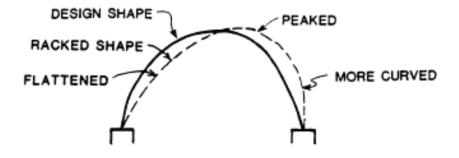
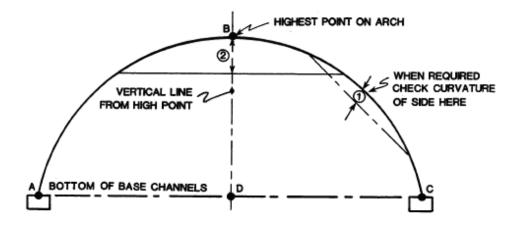




Figure 12.4.27 (Exhibit 88) Racked and Peaked Arch

Visual observation of the shape should involve looking for flattening of the sides, peaking or flattening of the crown, or racking to one side. The measurements to be recorded are illustrated in Exhibit 89. Minimum measurements include the vertical distance from the crown to the bottom of the base channels and the horizontal distances from each of the base channels to a vertical line from the highest point on the crown. These horizontal distances should be equal. When they differ by more than 10 inches or 5 percent of the span, whichever is less, racking has occurred and the curvature on the flatter side of the arch should be checked by recording chord and midordinate measurements. Racking can occur when the rise checks with the design rise. When the rise is more than 5 percent less than the design rise, the curvature of the top arc should be checked.



- 1. MINIMUM REQUIRED MEASUREMENTS
  - SPAN = AD + DC
  - RISE = BD
- 2. MINIMUM REQUIRED ELEVATIONS B
- 3. IF BD GREATER THAN DESIGN BY 5% OR MORE CHECK SIDE CURVATURE
- 4. IF AD AND DC NOT EQUAL CHECK SIDE CURVATURE (1)
- IF BD LESS THAN DESIGN BY 5% OR MORE CHECK TOP CURVATURE (2)

Figure 12.4.28 (Exhibit 89) Shape Inspection Structural Plate Arch

Arches in fair to good condition will have the following characteristics: a good shape appearance with smooth and symmetrical curvature, and a rise within three to four percent of theoretical. Marginal condition would be indicated when the arch is significantly non-symmetrical, when arch height is five to seven percent less or greater than theoretical, or when side or top plate flattening has occurred such that the plate radius is 50 to 100 percent greater than theoretical. Arches in poor to critical condition will have a poor shape appearance including significant distortion and deflection, extremely non-symmetrical shape, severe flattening (radius more than 100 percent greater than theoretical) of sides or top plates, or a rise more than eight percent greater or less than the theoretical rise. Guidelines for rating structural plate arches are shown in Exhibit 90.

As a starting point, select the lowest rating which matches actual conditions.

RATING	CONDITION	RATING	CONDITION
,		(A)	Chance
6	New condition	•	sides flattened with radius 100
œ	<ul> <li>Shape: good, smooth symmetrical curvature</li> <li>Rise: within + 3 percent of design</li> </ul>		than design  • Rise: within 7 to 8 percent of design
	Seams: properly made and tight     Matal: minor defects and damage due to contraction	2 6	Seams: major cracking of seam near crown; infiltration of soil causing major deflection
		10	• Metal:
*	<ul> <li>Steel: superficial rust, no pitting</li> <li>Footings: good with no erosion</li> </ul>		- Aluminum: extensive corrosion, significant attack of core alloy
•			- Steel: extensive heavy rust, deep pitting - Engines: rotated due to engine and undercutting: cettlement
-	des in one section		
	e Rise: within 3 to 4 percent of design		• Shape: poor, extreme distortion and deflection in one section;
		i i	sides virtually flattened; extremely non-symetrical
	e Hetal:	90	e Rise: within 8 to 10 percent of design
	- Steel: moderate rust, slight pitting	5	• Retal:
	• Footings: moderate erosion causing differential settlement and		- Aluminum: extensive corrosion, attack of core alloy,
	minor cracking in footing	1 5 6 1 3 1	scattered perforations - Steel: extensive heavy rust, deep pitting, scattered
9	• Shape: fair, smooth curvature but non-symetrical; slight	(1 :	
	flattening of top and sides throughout		• COULING: FOLACED, SEVERELY UNDEFCUE; MAJOR CFACKING AND SPAINING
		2	• Shape: critical, extreme deflection, throughout; sides
	evidence of backfill infiltration	01	flattened; extremely non-symetrical
	Wetal:		A Name: greater than 10 percent or design
	moderate pitting		backfill infiltration
	• Footings: moderate cracking and differential settlement of	10	• Metal:
	rooting due to extensive erosion	8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8 8	Steel: extensive perforations due to rust
s	• Shape: generally fair, significant distortion and deflection in		• Footing: severe differential settlement has caused distortion
	one section; sides beginning to Flattend; non-symetrical		and Kinking of metal arti
		2 9 n 3 A	e Shape: severe due to partial collapse; local reverse curve of
	of soil causing slight deflection		Crown and sides  Crown and sides
	- Aluminum: significant corrosion, moderate attack of core	SI F	• Road: closed to traffic
	alloy - Steal: coattered heavy rust, deep pitting	0	Structure: completely collapsed
	• Footings: significant undercutting of footing and extreme	75	
	differential settlement; major cracking in rooting	10	

Figure 12.4.29 (Exhibit 90) Condition Rating Guidelines

12.4.37

## 5-5.5 Corrugated Metal Box Culverts.

The box culvert is not like the other flexible buried metal structures. It behaves as a combination of ring compression action and conventional structure action. The sides are straight, not curved and the plates are heavily reinforced and have moment or bending strength that is quite significant in relation to the loads carried.

The key shape factor in a box culvert is the top arc. The design geometry is clearly very "flat" to begin with and therefore cannot be allowed to deflect much. The span at the top is also important and cannot be allowed to increase much.

The side plates often deflect slightly inward or outward. Generally an inward deflection would be the more critical as an outward movement would be restrained by soil.

Shape factors to be checked visually include flattening of top arc, outward movement of sides, or inward deflection of the sides. The inspector should note the visual appearance of the shape and should measure and record the rise and the horizontal span at the top of the straight legs as shown in Exhibit 91. If the rise is more or less than 1 ½ percent of the design rise, the curvature of the large top radius should be checked.

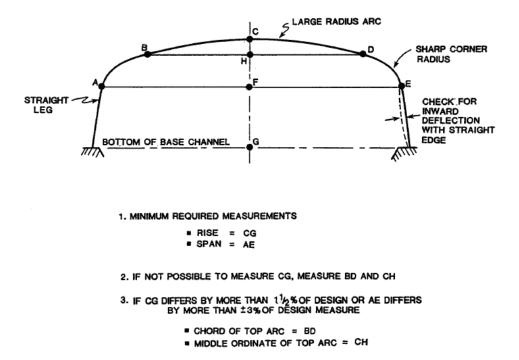


Figure 12.4.30 (Exhibit 91) Shape Inspection Structural Plate Box Culverts

The radius points are not necessarily located at the longitudinal seams. Many box culverts use double radius plates and the points where the radius changes must be estimated by the inspector or can be determined from the manufacturer's literature. These points can still be referenced to the bolt pattern to describe exactly where they are. Since these are all low structures, the spots should also be marked and

SECTION 12: Special Bridges TOPIC 12.4: Flexible Culverts

painted for convenient repeat inspection.

Box culverts in fair to good condition will appear to be symmetrical with smooth curves, slight or no deflection of the straight legs, a horizontal span length within five percent of the design span and the middle ordinate of the tops are within ten percent of the design. Culverts in marginal condition may appear to be non-symmetrical, have noticeable deflection in the straight legs, have spans that differ from design by five percent, or have a middle ordinate of the top arc that differ from design by 20 to 30 percent. Poor to critical conditions exist when the culvert shape appears poor, the culvert has severe deflections of the straight legs, a horizontal span that differs from design by more than five percent, or a middle ordinate of the top arc that differs from the theoretical by more than 40 to 50 percent. Guidelines for rating structural plate box culverts are shown in Exhibit 92.

As a starting point, select the lowest rating which matches actual conditions.

See Coding Guide for description of Rating Scale.

NOTES:

RATING	RATING GUIDELINES FOR CORRUGATED METAL BOX CULVERT BARREL	LVERT BARRE	;
RATING	CONDITION	RATING	CONDITION
	Shage: good appearance, smooth symmetrical curvature     Lipp Arc Mid-Ordinate: within 11 percents of design     Lipp Arc Mid-Ordinate: within 11 percents of design     Morizontal Span: within 3 percent of design     Morizontal Span: within 5 percent of design     Side: straight leg very stightly deflected inward or outward     Stams: properly made and tight     Morizontal minor defects and damage due to construction     Musimum: superficial corrosion, slight pitting     Side: superficial rust, no pitting     Side: superficial rust, no pitting     Simps: generally good: curvature is smooth and symmetrical	•	Shage: marginal, significant distortion and deflection throughout; mid-ordinate of half top arc less than 50 percent of design of the Arc Mid-Ordinate; within 20 to 30 percent of design of 100 Arc Mid-Ordinate; within 20 to 30 percent of design of 100 Arc Mid-Ordinate; within 20 to 30 percent of design of 100 Arc Mid-Ordinate; within 20 to 30 percent of design of 100 Arc Mid-Ordinate; within 100 Arc Mid-Ordinate of 100 Arc Mid-Ordinat
	2 2 4 2 4 - 0 - 6 8		
•	• Shape: smooth curvature, shape is non-symetrical  • Top Arc Mid-Ordinate: within 15 percent of design  • Mode: staraight leg moderately deflected inward or extremaly deflected outward, curvature smooth  • Saems: minor cracking at boil holes along one seam; evidence of backfill infiltration  • Metal:  • Aluminum: significant corrosion, minor attack of core alloy  • Sobalist fairly heavy rust, moderate pitting  • Footing: differential settlement due to extensive erosion; moderate cracking of footing	~	1 ' 8 g g g g g g . ' '
•	• Shage; generally fair; significant distortion and deflection in one section; half top arcs beginning to flatten; mid-ordinate of half top arc 30 percent less than design • 100_Arc Hid-Ordinate; within 15 to 20 percent of design • Morizonial, Spair; more than a or - 5 percent of design • 51dgs; straight leg bowed inward significantly or extremely bowed outland for distance of less than 1/4 span length • £egg; major cracking in one location; infiltration of soil causing silpht deflection		1/2 to 1 span length, or leg bowed outward severely causing busings or kinking in metal  • Sewings cracked from bolt to bolt; significant amounts of backfill infiltration throughout  • Metal:  • Metal:  • Metal:  • Steel: extensive perforations due to corrosion  • Steel: extensive perforations due to rust  • Steel: extensive perforations due to rust  • Gooting: severe differential settlement has caused distortion and kinking of metal arch.
	• Metal: - Aluminum: significant corrosion, moderate attack of core aloy - Steel: scattered heavy rust, deep pitting - Steel: scattered heavy rust, deep pitting - Egotings: significant undercutting of footing and extreme differential settlement; major cracking of footing	- •	• \$hage: severe due to partial collapse; top arc curvature flat or reverse curved • <u>Seans;</u> falled, backfill pushing in • <u>Road closed to traffic</u> • <u>\$tructure</u> : completely collapsed

Figure 12.4.31 (Exhibit 92) Condition Rating Guidelines

The following excerpts are from a reproduction of the out-of-print <u>Culvert</u> Inspection Manual (Supplement to Manual 70), July 1986 – Chapter 5, Section 6.

### Section 6. CORRUGATED METAL LONG-SPAN CULVERTS

#### 5-6.0 General.

This section describes procedures for conducting shape inspections of long-span structures. The long-span structures addressed include four typical shapes: low profile arch, horizontal ellipse, high profile arch, and pear. These shapes are illustrated in Exhibit 93. The evaluation of shape characteristics of long-spans will vary somewhat depending upon the typical shape being inspected. However, the top or crown sections of all long-span structures have very similar geometry. The crown sections on all long-span structures can be inspected using the same criteria. This section therefore includes separate discussions on the crown section and on each of the typical long-span shapes. Guidelines are also provided for rating the condition of each shape in terms of shape characteristics and barrel defects. The procedures for using the rating guidelines are the same as those described in section 5-5.1.

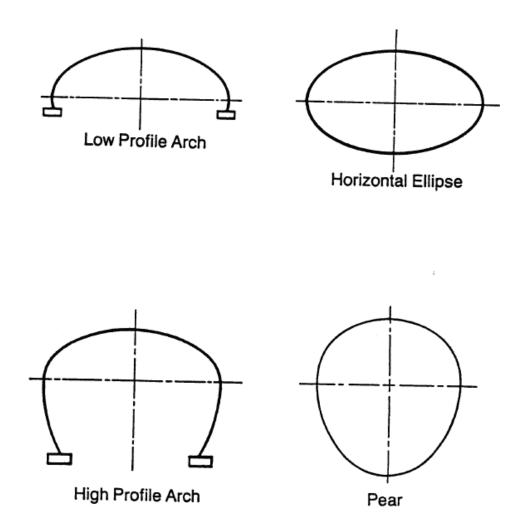


Figure 12.4.32 (Exhibit 93) Typical Long-Span Shapes

Shape inspections of long-span structures will generally consist of 1) visual observations of shape characteristics such as smooth or distorted curvature and symmetrical or non-symmetrical shape, 2) measurements of key dimensions, and 3) elevations of key points. Additional measurements may be necessary if measurements or observed shape differ significantly from design.

The visual observations are extremely important to evaluate the shape of the total cross section. Simple measurements such as rise and span do not describe curvature, yet adequate curvature is essential, as shown in Exhibit 94. However, measurements and elevations are also needed to document the current shape so that the rate change, if any, can be monitored.

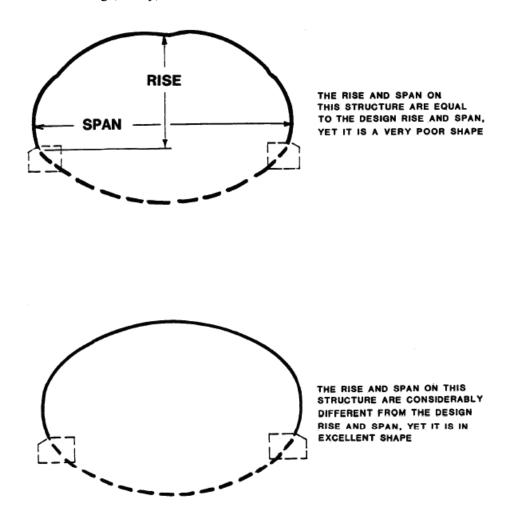


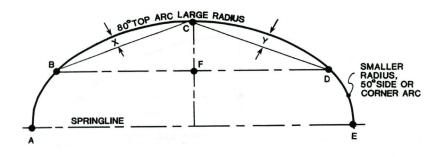
Figure 12.4.33 (Exhibit 94) Erosion Damage to Concrete Invert

Many long-spans will be too large to allow simple direct measuring. Vertical heights may be as large as 6.1 to 9.1 m (20 to 30 feet) and horizontal spans may be large and as high as 3.7 to 4.6 m (12 to 15 feet) above inverts. Culverts may have flowing water obscuring the invert and any reference points there. It is, therefore, in general desirable to have instrument survey points, which can be quickly checked for elevation. When direct measuring is practical a 7.6 m (25 foot) telescoping extension rod can be used for measuring. Such rods can also serve as level rods for taking elevations.

## 5-6.1 Long-Span Crown Section - Shape Inspection.

As previously mentioned, the section above the springline is essentially the same for most long-span shapes. With the exception of pear shapes, the standard top geometry uses a large radius top arc of approximately 80 degrees with a radius of 4.6 to 7.6 m (15 to 25 feet). The adjacent corner or side plates are from one-half to one-fifth the top arc radius. The most important part of a long-span shape is the standard top arch geometry. Adequate curvature of the large radius top arc is critical. Inspection of the crown section should consist of a visual inspection of the general shape for smooth curvature (no distortion, flattening, peaks, or cusps) and symmetrical shape (no racking).

An inspection should also include key measurements such as the middle ordinate of the top arc. Recommended measurements and elevations are shown in exhibit 95.



1. MINIMUM REQUIRED ELEVATIONS - B, C, D

MINIMUM REQUIRED MEASUREMENTS 
■ TOP SPAN = AE

CALCULATE CF = ELEV C - ELEV B + ELEV D

2

2. IF CF IS GREATER THAN OR LESS THAN DESIGN BY 10% MEASURE:

■ TOP ARC CHORD = BD

3. IF BD DIFFERS BY MORE THAN 3% FROM DESIGN MEASURE FOR EACH HALF OF TOP ARC

■ HALF TOP ARC MID ORDINATES = X & Y

Note: These measurements and elevations should be obtained on all long span inspections (see exhibits 96, 98, 100 and 103).

**Figure 12.4.34 (Exhibit 95)** Shape Inspection Crown Section of Long Span Structures

The initial inspection should establish elevations for the radius points and the top of the crown. From these elevations the middle ordinate for the top arc can be calculated. If the actual middle ordinate is 10 percent more or less than the theoretical design mid-ordinate the horizontal span for the top arc should also be measured. For standard 80 degree arcs the theoretical middle ordinate is equal to 0.234 times the theoretical radius of the top arc. This span is not easy to measure on many long-span structures and need not be measured if the top arc mid-ordinate is within 10 percent of theoretical. Even if it is convenient and practical to direct measure the vertical heights of the points on the top arc from the bottom of the structure, it is wise to also establish their elevations from a reliable benchmark. Bottom reference points can be wiped out by erosion, covered with debris, or covered by water. When direct vertical measuring is practical, the shape may be checked on subsequent inspections with direct measurement. However, it is still important to establish elevations in case bottom reference points are lost or inaccessible.

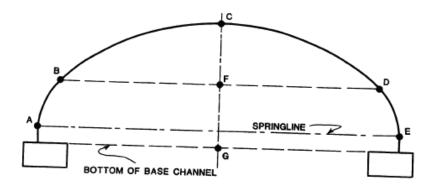
Crown sections in good condition will have a shape appearance that is good, with smooth and symmetrical curvature. The actual middle ordinate should be within 10 percent of the theoretical, and the horizontal span (if measured) should be within five percent of theoretical. Crown sections in fair condition will have a fair to good shape appearance, smooth curvature but possibly slightly non-symmetrical. Middle ordinates of the top arc may be within 11 to 15 percent of theoretical and the horizontal span may differ by more than 5 percent of theoretical.

Crown sections in marginal condition will have measurements similar to those described for fair shape. However, the shape appearance will be only fair to marginal with noticeable distortion, deflection, or non-symmetrical curvature. When the curvature is noticeably distorted or non-symmetrical, the sides should be checked for flattening by measuring the middle ordinates of the halves of the top arc. Crown sections with marginal shape may have middle ordinates for top half arcs that are 30 to 50 percent less than theoretical.

Crown sections in poor to critical condition will have a poor to critical shape appearance with severe distortion or deflection. The middle ordinate of the top arc may be as much as 20 percent less than theoretical, while middle ordinates of the top arc halves may be 50 to 70 percent less than theoretical.

### 5-6.2 Low Profile Long-Span Arch - Shape Inspection.

The low profile arch is essentially the same as the crown section except that the sides are carried about 10 degrees below the springline to the footing. These structures are low and can be measured more easily than other long-span shapes. Recommended measurements and elevations are shown in exhibit 96. Rating guidelines are listed in exhibit 97.



AE = SPAN, CG = RISE OR HEIGHT

- 1. MINIMUM REQUIRED MEASUREMENTS -
  - SPAN = AE
  - TOP ARC CHORD = BD
  - RISE = CG
- 2. MINIMUM REQUIRED ELEVATIONS B, C, D
- 3. CALCULATE CF FROM ELEVATIONS

Note: Use with exhibit 95, crown inspection.

Figure 12.4.35 (Exhibit 96) Shape Inspection Low Profile Long Span Arch

RATING	CONDITION	RATING	CONDITION
_	New condition	T U9	Shage: marginal, significant distortion and deflection throughout; mid-ordinate of half top are less than 50 percent
_			of design
	- Horizontal Span: within 5 percent of design	16	- 100 Arc nig-Urainate: Within 13 to 20 percent of design - Horizontal Span: more than + or - 5 percent of design
915.	<ul> <li>Seang: properly made and tight</li> <li>Metal: minor defects and damage due to construction</li> </ul>		<ul> <li>Seans: significant seam cracking all along seam; infiltration of soil causing major deflection</li> </ul>
1797	- Aluminum: superficial corrosin, slight pitting	60 V 1 I	• Metal: - Aluminum: extensive corrosion, significant attack of core
9794	· Footings: good with no erosion		Alloy.
00	e Shape: generally good; curvature is smooth and symmetrical		• Footings: rotated due erosion and undercutting; settlement has
37	- Iop Arc Mid-Ordinate: within II percent to 15 percent of	1	caused damage to metal arch
	- Horizontal Span: within 5 percent of design	•	. Shape: poor extreme distortion and deflection in one section
91	· Seans: minor cracking at a few bolt holes; minor seam openings,	1 2 2	and ordinate of half top arc 50 to 10 percent less than design
	possibility of backfill infiltration exists.		- Top Arc Mid-Ordinate: 20 to 30 percent less than design
	• Retal:	1 G	<ul> <li>Ilorizontal Span: more than + or - 6 percent of design</li> <li>Seans: cracked 3" or more to either cide of holt: infiltration</li> </ul>
			of backfill causing severe deflection locally
1	· Footings: minor differential settlement due to erosion; minor		• Kul:
) 1	Natriine cracking in footing	10	scattered perforations
5	Shape: smooth curvature, shape is non-symetrical     Too Arc Mid-Ordinate: within 15 percent of decion	\$ 1 to 9	- Steel: extensive heavy rust, deep pitting, scattered perforations
ș.	- Horizontal Span: more than + or - 5 percent of design	D. 011	· Footing: rotated, severely undercut, major cracking and
	<ul> <li>Seams: minor cracking at boil holes along one seam; evidence of backfill infiltration</li> </ul>		spalling of footing, significant damage to structure
	• Metal:	2	• Shape: critical, extreme distortion and deflection throughout;
10	- Steel: fairly heavy rust, moderate pitting		design
	· Footing: differential settlement due to extensive erosion;	1	- Top Arc Mid-Ordinate: more than 30 percent less than design
10	independent of the state of the		Seams: cracked from bolt to bolt: significant amounts of
F	. Shape: generally fair; significant distortion and deflection in	3	backfill infiltration throughout
- Ale	one section; half top arcs beginning to flatten; mid-ordinate of	5 0 5	A luminum authorities and formation due to
Ł	- Top Arc Mid-Ordinate: within 15 to 20 percent of design		- Steel: extensive perforations due to rust
7	or - S percent of design		· Footing: severe differential settlement has caused distortion
0	· Seans: major cracking in one location; infiltration of soil	1 6	and kinking of metal arch
	e Metal:		. Shape: severe due to partial collapse: too arc curvature flat
1	- Aluminum: significant corrosion, moderate attack of core	10	or reverse curved
	- Steel: scattered heavy rust, deep bitting		Road closed to traffic
ia.	· Footings: significant undercutting of footing and extreme		
7	differential settlement; major cracking of footing	0	• Structure: completely collapsed

Figure 12.4.36 (Exhibit 97) Condition Rating Guidelines

See Coding Guide for description of Rating Scale. NOTES:

a starting point, select the lowest rating which matches actual conditions.

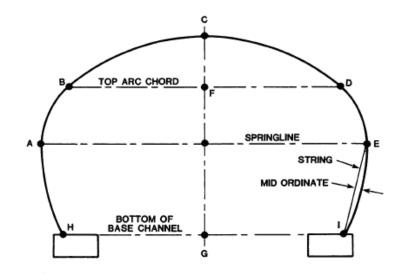
Because arches are fixed on concrete footings, backfill pressures will try to flatten the sides and peak the top. Another important shape factor is symmetry. If the base channels are not square to the centerline of the structure racking may occur during erection. In racked structures, the crown moves laterally and the curvature in one side becomes flatter while the curvature in the other side increases. Backfill pressures may cause this condition to worsen.

#### 5-6.3 High Profile Long-Span Arch – Shape Inspection.

High profile arches have a standard crown section geometry but have high large radius side walls below the springline. Curvature in these side plates is important. In shallow fills or minimum covers, the lateral soil pressures may approach or exceed the loads over the culvert. Excessive lateral forces could cause the sidewall to flatten or buckle inward.

Inspectors should visually inspect high profile arches for flattening of the side plates. Additionally, high profile arches have the same tendencies as regular arches for peaking and racking, so inspectors must also look for peaked top arcs and non-symmetrical or racked arches.

Recommended measurements and elevations are shown in Exhibit 98. The shape of the crown section is the most important shape factor. It can be measured and evaluated using the same criteria as that described for the standard crown section. If flattening is observed in the high sidewall the curvature of the sides should be checked by measuring the middle ordinate of the side walls. If the sidewall middle ordinate is no more than 50 to 70 percent less than the theoretical middle ordinate and no other shape problems are found the arch's shape may be considered fair. When the middle ordinate approaches 75 to 80 percent less than theoretical, the shape should be considered marginal. If the middle ordinate is more than 80 to 90 percent less than theoretical the shape should be considered poor to critical. Rating guidelines are provided in Exhibit 99.



AE = SPAN, CG = RISE

1. MINIMUM REQUIRED MEASUREMENTS

• SPAN = AE

2. MINIMUM REQUIRED ELEVATIONS - B, C, D, H, I

3. CALCULATE CF FROM ELEVATIONS

Note: Use with exhibit 95, crown inspection.

Figure 12.4.37 (Exhibit 98) Shape Inspection High Profile Long-Span Arch

t				T
	CONDITION	RATING	CONDITION	
	New condition	•	<ul> <li>Shage: morginal, significant distortion and deflection throughout; mid-ordinate of half top arc less than 50 percent</li> </ul>	
	• Shape: good appearance, smooth symmetrical curvature  - Top Arc Nid-Ordinase: within 11 percent of design  - Morizontal Span: within 5 percent of design		of design  - Jop Arc Mid-Ordinale: within 15 to 20 percent of design  - Uprizontal Spain more than + or - 5 percent of design  - the Datase: the flattened mid-ordinale less than 20	
			percent of design percent of design sing: significant sean cracking soil causing major deflection tal:	
	- Siee; superities rust, no pitting  Footing: good with no erosion  Chane menerally mode curvature is smooth and symmetrical			
	100 Are bid-Ordinate: within 11 percent to 15 percent of design		<ul> <li><u>Exclings</u>: rotated due erosion and undercutting; settlement has caused damage to metal arch</li> </ul>	
_	<ul> <li>Horizontal Span; within 5 percent or design</li> <li>Fide Plates: side flattened, mid-ordinate less than 50 percent of design</li> </ul>	•	<ul> <li>Shape: poor extreme distortion and deflection in one section and ordinate of half top arc 50 to 10 percent less than design</li> </ul>	
	<ul> <li>Seams: minor cracking at a few bolt holes; minor seam openings, possibility of backfill infiltration exists</li> </ul>		<ul> <li>100 Arc Mid-Ordinate: 70 to 30 percent less than design</li> <li>- Morizonte 1 Spain more than + or - 6 percent of design</li> <li>- Side plates: side flattened, mid-ordinate less than 12</li> </ul>	
	- Aluminus: moderate corrosion, no attack of core alloy - Steel: moderate rust, slight pitting		percent of design establisher side of bolt; infiltration	
	• FOOLINGS: minor differential settlement due to erosion; minor hairline cracting in footing			
	• Shape: smooth curvature, shape is non-symetrical		- Aluminum: extensive corrosion, attack of core alloy, scattered perforations	
	- T <u>op Arc Mid-Ordinate; vithin 15 percent of design</u> - <u>Hortzontal Spair</u> more than + or - 5 percent of design - <u>Sider Dates</u> ; side flattened, mid-ordinate less than 35		alestic avenive meny rast, over pitting, scattered perforations e [coling: rotated, severely undercut, major cracking and	
	percent of design • Seams: minor cracking at bolt holes along one seam; evidence of	•	spaining of footing, significant damage to structure a Chana critical astrong distance for and deflaction throughout	
	backfill infiltration • Metal:			
	- Aluminus: significant corrosion, minor attack of core alloy - Steel: fairly heavy rust, moderate pitting - Footing: differential settlement due to extensive erosion; moderate cracking of footing		- 100 Arc. Mid-Ordinate; more than 30 percent less than,design - 100 Arc. Mid-Ordinate; more then + or - 8 percent of design - 51de Flattered, mid-ordinate less than lopercent of design.	
	<ul> <li>Shape: generally fair: significant distortion and deflaction in one section;half top arcs beginning to flatten; mid-ordinate of</li> </ul>		• Same: creeked from boil to boil; significant amounts of backfull infiltration throughout	
	half top arc 30 percent less than design  - Top Arc Ald-Ordinage: within 15 to 20 percent of design  Hartrooks Coan more than a or 5 percent of design		- Steel extensive perforations due to corrosion - Steel extensive perforations due to rust	-
			<ul> <li>Footing: severe differential settlement has caused distortion and kinking of metal arch</li> </ul>	
	• Searg: major cracking in one location; Infiltration of soil causing slight deflection	-	Shage: severe due to partial collapse; top arc curvature flat     no reserve curvad	
_	- Aluminus: significant corrosion, moderate attack of core alloy		- Side Plates: side flat or reversed curved	
	- Steel: scattered heavy rust, deep pitting e Footings and extreme		Road closed to traffic	
	differential settlement: major cracking of footing	•	· Structure: completely collapsed	

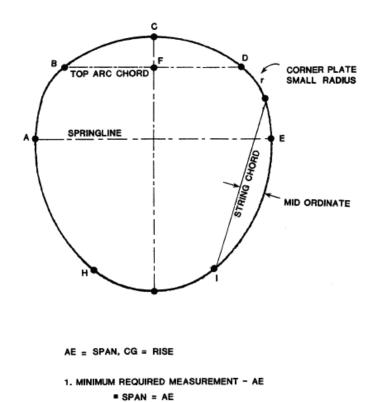
Figure 12.4.38 (Exhibit 99) Condition Rating Guidelines

As a starting point, select the lowest rating which matches actual conditions. See Coding Guide for description of Rating Scale. NOTES:

#### 5-6.4 Pear Shape Long-Span – Shape Inspection.

The crown section of the pear shape differs from the standard top arch in that smaller radius corner arcs stop short of the horizontal springline. The large radius sides extend above the plane of the horizontal span. In checking curvature of the sides, the entire arc should be checked. Side flattening, particularly in shallow fills, is the most critical shape factor.

The pear shape behaves similarly to the high profile arch. It is essentially a high profile with a metal bottom instead of concrete footings. Pears may be inspected using the criteria for a high profile arch. The recommended measurements and elevations are shown in Exhibit 100. Rating guidelines are provided in Exhibit 101.



2. MINIMUM REQUIRED ELEVATIONS B, C, D

3. WHEN FLATTENING OBSERVED IN SIDE, CHECK MID ORDINATE (RECORD CHORD LENGTH USED)

Note: Use with exhibit 95, crown inspection.

Figure 12.4.39 (Exhibit 100) Shape Inspection Long Span Pear-Shape

As a starting point, select the lowest rating which matches actual conditions.

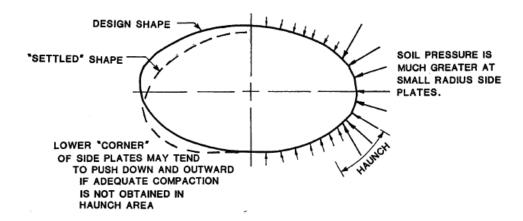
See Coding Guide for description of Rating Scale.

SATING	RATING GUIDELINES FOR PEAR SHAPED LONG-SPAN CULVERT BARREL	ULVERT BARF	REL
RATING	CONDITION	RATING	CONDITION
•	Mew condition	•	Shape: marginal, significant distortion and deflection throughout; mid-ordinate of half top arc less than 50 percent.
•	<ul> <li>Shape: good appearance, smooth symmetrical curvature</li> <li>Ing Arc Mid-Ordinate: within 11 percent of design</li> <li>Morizontal Span: within 5 percent of design</li> </ul>		of design - 100 Arc Mid-Ordinate: within 15 to 20 percent of design - 100 fac list states are than a cross of section - 1001720013 Scans: more than a cross of section
	Plates: smooth curvature properly made and tight		- Side Plates: side flattened, mid-ordinate less than 20 percent of design
1	<ul> <li>Metal: whor defects and damage due to construction; superficial corrosion with no pitting</li> <li>Aluminus; superficial corrosion, silght pitting</li> </ul>		<ul> <li><u>Seems</u>: significant seam cracking all along seam; infiltration of soil causing major deflection</li> <li>Massing major deflection</li> </ul>
,744 ,310	- <u>Steel;</u> superficial rust, no pitting		- Aluminum: extensive corrosion, significant attack of alloy - Steel: extensive heavy rust, deep pitting
	<ul> <li><u>Shape</u>: generally good; curvature is smooth and symmetrical         - <u>lop Arc Mid-Ordinate</u>: within II percent to 15 percent of         declarate.</li> </ul>		• <u>Shape</u> : poor extreme distortion and deflection in one section
	- Horizongal Span: Within 5 percent of design - Kide Plates: side Platened, mid-ordinate less than 50	9.1	and ordinate of half top arc 50 to 10 percent less than design - Top Arc Mid-Ordinate: 30 to 30 percent less than design
	percent of design	The second secon	- Horizontal Span: more than + or - 6 percent of design - Side Plates: side flattened, mid-ordinate less than 12
	<ul> <li>Seams: minor cracking at a few bolt holes; minor seam openings, possibility of backfill infiltration exists</li> </ul>		bolt; infiltrat
	- Aluminum: moderate corrosion, no attack of core alloy		lusing severe deflection
	בובניי שחספו פוע וחסיי סיותור לייייים		- Aluminum: extensive corrosion, attack of core alloy, scattered perforations
•	Shape: smooth curvature, shape is non-symetrical     Top Are Mid-Ordinate; within is percent of design     Horizontal Spain: more than + or - 5 percent of design		- Siegi: extensive heavy rust, deep pitting, scattered perforations
	- Side Plates: side flattened, mid-ordinate less than 35 percent of design	~	<ul> <li>Shape: critical, extreme distortion and deflection throughout; mid-ordinate of half ton are more than 10 necessit lass than</li> </ul>
	Seams: minor cracking at bolt holes along one seam; evidence of     backfill infiltration		design
	Netal:     Aluminum: significant corrosion, minor attack of core alloy     Steel: fairly heavy rust, moderate pitting		- Side Plates: side flattened, mid-ordinate less than Opercent . Side Plates: side flattened, mid-ordinate less than lopercent .
<b>S</b>	• Shape: generally fair; significant distortion and deflection in		or usasym  • <u>Seams</u> : cracked from bolt to bolt; significant amounts of backfill infiltration throughout
	one section; half top arcs beginning to flatten; mid-ordinate of half top arc 30 percent less than design		Ministral:
	- Top Arc Mid-Ordinate: within 15 to 20 percent of design - Horizontal Span: more than + or - 5 percent of design		Steel: extensive perforations due to rust
	- Side Plates: side flattened, mid-ordinate less than 25 percent of design	-	• <u>shape</u> : severe due to partial collapse; top arc curvature flat
	e Seans: major cracking in one location; infiltration of soil causing slight deflection	1.4	or reverse curved - Side Plates: side flat or reversed curved
	Metal: corroded locally     Aluminum: significant corrosion, moderate attack of core		Soad closed to traffic
	- Steel: scattered heavy rust, deep pitting	0	e Structure; completely collapsed
			Word: Closed to traffic

Figure 12.4.40 (Exhibit 101) Condition Rating Guidelines

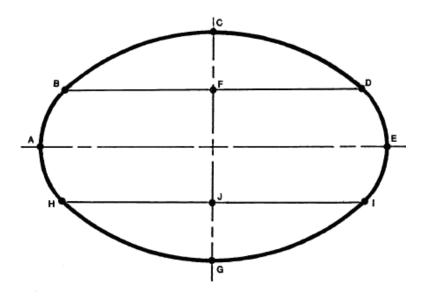
#### 5-6.5 Horizontal Ellipse – Shape Inspections.

For horizontal ellipses the most important shape factor is adequate curvature in the crown section. The crown section uses the standard long-span crown geometry. The sides and bottom behave similar to the corners and bottom of pipe arches. The invert has relatively minor pressure when compared with the sides, which may have several times the bearing pressure of the invert. As a result the corners and sides have the tendency to push down into the soil while the bottom does not move. The effect is as if the bottom pushed up. Inspectors should look for indications of bottom flattening and differential settlement between the side and bottom sections, as illustrated in Exhibit 102.



**Figure 12.4.41 (Exhibit 102)** Potential for Differential Settlement in Horizontal Ellipse

The recommended measurements and evaluations for a shape inspection of horizontal ellipse are shown in Exhibit 103. The measurements are essentially the same as those recommended for a standard crown section. Shape evaluation of an ellipse is also essentially the same as the evaluation of a standard crown section except that the curvature of the bottom should also be evaluated. Marginal shape would be indicated when the bottom is flat in the center and corners are beginning to deflect downward or outward. Critical shape conditions would be indicated by reverse curvature in the bottom arc. Guidelines for rating horizontal ellipse shape culverts are provided in Exhibit 104.



- 1. MINIMUM REQUIRED MEASUREMENTS

  SPAN = AE
- 2. MINIMUM REQUIRED ELEVATIONS B, C, D, G (IF POSSIBLE)
- 3. WHEN BOTTOM FLATTENING IS OBSERVED, CHECK CURVATURE,
  MEASURE

■ BOTTOM ARC CHORD = HI ■ BOTTOM ARC MIDDLE ORDINATE = JG

Note: Use with exhibit 95, crown inspection.

Figure 12.4.42 (Exhibit 103) Shape Inspection Long-Span Horizontal Ellipse

a starting point, select the lowest rating which matches actual conditions.

RATING	CONDITION	RATING	CONDITION
•	• New condition	2	. Shapg: marginal, significant distortion and deflection
•	• <u>Shape:</u> good appearance, smooth symmetrical curvature - Iop Arc Hid-Ordinate: within 11 percent of design		throughout; mid-ordinate of half top are less than 50 percent of design — ron dre Mid-ordinate; within 15 to 20 percent of design
	- Horizontal Span: within 5 percent of design - Botton Arg: smooth curvature, aid-ordinate within 50 percent - Botton Arg: smooth curvature, aid-ordinate within 50 percent		- light light 5245; more than + or - 5 percent of design - Bottom Arg: bottom virtually flat over center half of arc
	or using:  • <u>sears;</u> properly made and tight  • Retal; minor defects and damage due to construction		and deflected down at corners  • <u>Sears</u> : significant sean cracking all along sean; infiltration
	- Steel: superficial corrosi		Of soil causing major deflection     Mala]:     Alembium: extensive corrosion, significant attack of alloy
,	<ul> <li>Fogting: good with no erosion</li> <li>Shape: generally good: curvature is smooth and symmetrical</li> </ul>	AS	- 1981: extensive heavy rust, deep pitting • Eggling: rotated due erosion and undercutting; settlement has
	- Top Arc Mid-Ordinate: within 11 percent to 15 percent of design	110	
	- Horizontal Span: within 5 percent of design - Bottom Arc: bottom flattened, mid-ordinate less than 50	I J	and ordinate of half top are 50 to 70 percent less than design
	percent of design		- 100 Arc Ald-Ordinais: 30 to 30 percent less than design - [lorizonia] Span: more than + or - 6 percent of design
	possibility of backfill infiltr		- Bollom Arg: bottom reverse curved in center of Seans: cracked 3" or more to either side of bolt; infiltration
	١ اع		of backfill causing severe deflection locally
	<ul> <li>Steel: moderate rust, slight pitting</li> <li>Footings: minor differential settlement due to erosion: minor</li> </ul>		- Aluminum: extensive corrosion, attack of core alloy,
		70	scattered perforations - signi: extensive heavy rust, deep pitting, scattered
9	e <u>Shape</u> : smooth curvature, shape is non-symetrical		perforations everely indercut major cracking and
	- Top Arc Hid-Ordinate: within 15 percent of design - Horizontal Span: more than + or - 5 percent of design		spalling of footing, significant damage to structure
	- Bottom Arc: bottom flattened and irregular, mid-ordinate	•	Shape: critical extreme distortion and deflection throughout:
	less than 50 percent of design a Coame came, evidence of		
			•
	• Metal: - Aluminum: simplificant correction minor attack of core allow		- 190 Arc nig-urginate: more than so percent less than design - [locistonte] Span: more than + or - 8 percent of design
	fairly heavy		- Bottom Arc: bottom reversed curved in center and bulged out
	<ul> <li><u>Fooling</u>: differential settlement due to extensive erosion; moderate cracking of footing</li> </ul>	199	Seans: Cracked from bolt to bolt; significant amounts of backfill inflication throughout
<b>~</b>	. Shape: generally fair; significant distortion and deflection in		• Helel: - Aluminum: extensive perforations due to corrosion
	half top arc 30 percent less than design		
	- Top Arc Mid-Ordinata: within 15 to 20 percent of design		<ul> <li>EQQLING: severe differential settlement has caused distortion and kinking of metal arch</li> </ul>
	- Bottom Arc: bottom virtually flat over center half of arc		
			Shape: savere due to partial collapse; top arc curvature flat     Or reverse curved
	• Metal: - Aluminum: stonificant corresion, moderate attack of core		e Road closed to traffic
		-	A Structure: completely collanced
	<ul> <li>Steel: scattered heavy rust, deep pitting</li> <li>Footings: significant undercutting of footing and extreme</li> </ul>	•	Road: closed to traffic

Figure 12.4.43 (Exhibit 104) Condition Rating Guidelines

#### 12.4.7

### **Evaluation**

State and federal rating guideline systems have been developed to aid in the inspection of flexible culverts. The two major rating guideline systems currently in use are the FHWA's Recording and Coding Guide for the Structural Inventory and Appraisal of the Nation's Bridges used for the National Bridge Inventory (NBI) component rating method and the AASHTO element level condition state assessment method.

#### **NBI Rating Guidelines**

Using NBI rating guidelines, a 1-digit code on the Federal Structure Inventory and Appraisal (SI&A) sheet indicates the condition of the culvert (Item 62). This item evaluates the alignment, settlement, joints, structural condition, scour, and other items associated with culverts. Rating codes range from 9 to 0 where 9 is the best rating possible. See Topic 4.2 (Item 62) for general descriptive codes and Topics 12.4.6 through 12.4.8 for specific codes for the various flexible culverts. The rating code is intended to be an overall evaluation of the culvert. Integral wingwalls to the first construction or expansion joint shall be included in the evaluation. It is also important to note that Items 58-Deck, 59-Superstructure, and 60-Substructure shall be coded "N" for all culverts.

The previous inspection data should be considered along with current inspection findings to determine the correct rating.

### **State Assessment**

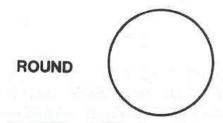
**Element Level Condition** In an element level condition state assessment of a flexible culvert, the AASHTO CoRe element is:

Element No.	<u>Description</u>
240	Unpainted Steel Culvert
243	Culvert: Other

The unit quantity for culverts is meters or feet of culvert length along the barrel. The total quantity equals the culvert length times the number of barrels. The inspector must visually evaluate each 1 m (1 ft) slice of the culvert barrel(s) and assign the appropriate condition state description. The total length must be distributed among the four available condition states depending on the extent and severity of deterioration. The sum of the individual condition state quantities must equal the total element quantity. Condition state 1 is the best possible rating. See the AASHTO Guide for Commonly Recognized (CoRe) Structural Elements for condition state descriptions.

A Smart Flag is used when a specific condition exists, which is not described in the CoRe element condition state. The severity of the damage is captured by coding the appropriate Smart Flag condition state. The Smart Flag quantities are measured as each, with only one each of any given Smart Flag per bridge.

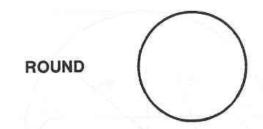
For settlement of the culvert, the "Settlement" Smart Flag, Element No. 360, can be used and one of three condition states assigned. For channel scour at the culvert ends, the "Scour" Smart Flag, Element No. 361, can be used and one of three condition states assigned.



Handling Weight of Corrugated Steel Pipe (21/3 × ½ in.) Estimated Average Weights—Not for Specification Use\*

Inside	Specified		Approximate P	ounds per Lineal Ft*	
Diameter, in.	Thickness, in,	Galvanized	Full- Coated	Full-Coated and invert Paved.	Full-Coated and Full Paved
12	0.052 0.064 0.079	8 10 12	10 12 14	13 15 17	
15	0.052 0.064 0.079	10 12 15	12 15 18	15 18 21	
18	0.052 0.064 0.079	12 15 18	14 19 22	17 22 25	
21	0.052 0.064 0.079	14 17 21	16 21 25	19 26 30	
24	0.052 0.064 0.079	15 19 24	17 24 29	20 30 35	45 50
30	0.052 0.964 0.079	20 24 30	22 30 36	25 36 42	55 60
36	0.052 0.064 0.079	24 29 36	26 36 43	29 44 51	65 75
42	0.052 0.054 0.079	28 34 42	30 42 50	33 51 59	85
48	0.052 0.064 0.079	31 38 48	33 48 58	36 57 67	95
54	0.064 0.079	44 54	55 65	66 76	95 105
60	0.079	60 81	71 92	85	2000000
66	0.109 0.138	89 113	101 125	106	160
72	0.109 0.138	98 123	112	141	180 170
78	0.109	105 133	121 149	154	21C 200
84	0.109	113 144	133 161	166 155 179	230 225
90	0.109 0.138 0.168	121 154 186	145 172 204	167 192 224	240
96	0.138 0.168	164 198	191 217	217 239	6

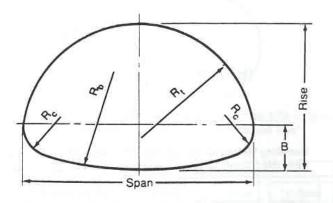
**Figure 12.4.44** Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute)



Handling Weight of Corrugated Steel Pipe (3 × 1 in. or 5 × 1 in.)\*\*\* Estimated Average Weights—Not for Specification Use

2000-909000			Approximate Po	unds per Lineal Ft**	
Inside Diameter, in.	Specified Thickness, in.	Galvanized	Full- Coated	Full-Coated and Invert Paved	Full-Coated and Full Paved
54	0.064	50	66	84	138
	0.079	61	77	95	149
60	0.064	55	73	93	153
	0.079	67	86	105	165
66	0.064	60	80	102	168
	0.079	74	94	116	181
72	0.064	66	88	111	183
	0.079	81	102	126	197
78	0.064	71	95	121	198
	0.079	87	111	137	214
84	0.064	77	102	130	213
	0.079	94	119	147	230
90	0.064	82	109	140	228
	0.079	100	127	158	246
96	0.064	87	116	149	242
	0.079	107	136	169	262
102	0.064	93	124	158	258
	0.079	114	145	179	279
108	0.064	98	131	166	273
	0.079	120	153	188	295
114	0.064	104	139	176	289
	0.079	127	162	199	312
120	0.064	109	146	183	296
	0.079	134	171	210	329
	0.109	183	220	259	378
126	0.079	141	179	220	346
	0.109	195	233	274	400
132	0.079	148	188	231	363
	0.109	204	244	287	419
138	0.079	154	196	241	379
	0.109	213	255	300	438
144	0.109	223	267	314	458
	0.138	282	326	373	517

**Figure 12.4.44** Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued



### Sizes and Layout Details—CSP Pipe Arches 245 × 1/2 in. Corrugation

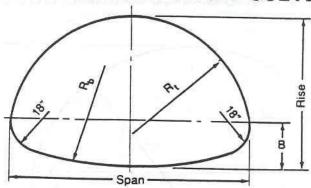
F10.00					Layout Dimensions					
Equiv. Diameter, in.	Span, in.	Rise. in.	Waterway Area, ft <sup>2</sup>	B in.	R <sub>c</sub> in.	R <sub>t</sub> in.	R <sub>b</sub> in.			
15	17	13	1.1	41/8	31/2	85%	25%			
18	21	15	1.6	47/8	41/8	103/4	331/8			
21	24	18	2.2	5%	47/8	117/8	345%			
24	28	20	2.9	61/2	51/2	14	421/4			
30	35	24	4.5	81/8	67/8	17%	551/6			
36	42	29	6.5	93/4	81/4	211/2	661/8			
42	49	33	8.9	113/8	95%	251/8	771/4			
48	57	38	11.6	13	11	285/8	881/4			
54	64	43	14.7	145%	123/8	321/4	991/4			
15 18 21 24 30 36 42 48 54 60 66	71	47	18.1	161/4	133/4	353/4	1101/4			
66	77	52	21.9	177/8	151/8	39%	1211/4			
72	83	52 57	26.0	191/2	161/2	43	1321/4			

Dimensions shown not for specification purposes, subject to manufacturing tolerances.

### Sizes and Layout Details—CSP Pipe-Arches $3 \times 1$ in. Corrugation

Cardio				We a		Layout D	imensions	
Equiv. Diameter. in.	Size, in.	Span, in.	Rise, in.	Waterway Area, ft <sup>2</sup>	B in.	R <sub>c</sub> in.	R <sub>t</sub> in.	R <sub>b</sub> in.
54	60 × 46	581/2	481/2	15.6	201/2	18¾	293/8	511/8
60	$66 \times 51$	65	54	19.3	223/4	203/4	325/8	561/4
66	$73 \times 55$	721/2	581/4	23.2	251/8	221/8	363/4	633/4
72	$81 \times 59$	79	621/2	27.4	233/4	20%	391/2	825%
78	$87 \times 63$	861/2	671/4	32.1	253/4	225/8	433/8	921/4
84	$95 \times 67$	931/2	713/4	37.0	273/4	243/8	47	1001/4
78 84 90 96	$103 \times 71$	1011/2	76	42.4	293/4	261/8	511/4	1115%
	$112 \times 75$	1081/2	801/2	48.0	31%	273/4	547/8	1201/4
102	$117 \times 79$	1161/2	843/4	54.2	33%	291/2	59%	1313/4
108	$128 \times 83$	1231/2	891/4	60.5	35%	311/4	631/4	1393/
114	$137 \times 87$	131	933/4	67.4	375/8	33	673/8	1491/2
120	$142 \times 91$	1381/2	98	74.5	391/2	343/4	71%	1623/

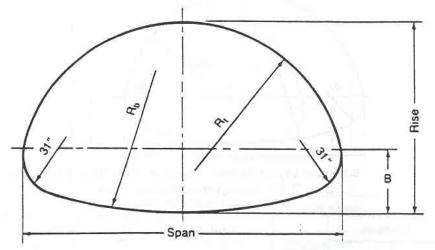
Figure 12.4.44 Standard Sizes for Corrugated Steel (Source: American Iron and Steel Institute), continued



Sizes and Layout Details—Structural Plate Steel Pipe-Arches
6 × 2 in. Corrugations—Bolted Seams
18-inch Corner Radius Rc

Dim	ensions		La	yout Dimen	isions	F	eriphery	
Span,	Rise,	Waterway Area,	В	Rt	R <sub>b</sub>	No. of	To	tai
ft-in.	ft-in.	ft²	in.	ft	ft	Plates	N	Pi
6-1 6-4 6-9 7-0 7-3	4-7 4-9 4-11 5-1 5-3	22 24 26 28 31	21.0 20.5 22.0 21.4 20.8	3.07 3.18 3.42 3.53 3.63	6.36 8.22 6.96 8.68 11.35	5 5 5 5 6	22 23 24 25 26	66 69 72 75 78
7-8 7-11 8-2 8-7 8-10	5-5 5-7 5-9 5-11 6-1	33 35 38 40 43	22.4 21.7 20.9 22.7 21.8	3.88 3.98 4.08 4.33 4.42	9.15 11.49 15.24 11.75 14.89	6 6 7 7	27 28 29 30 31	81 84 87 90 93
9-4 9-6 9-9 10-3 10-8	6-3 6-5 6-7 6-9 6-11	46 49 52 55 58	23.8 22.9 21.9 23.9 26.1	4.68 4.78 4.86 5.13 5.41	12.05 14.79 18.98 14.86 12.77	7 7 7 7	32 33 34 35 36	96 99 102 105 108
10-11 11-5 11-7 11-10 12-4	7-1 7-3 7-5 7-7 7-9	61 64 67 71 74	25.1 27.4 26.3 25.2 27.5	5.49 5.78 5.85 5.93 6.23	15.03 13.16 15.27 18.03 15.54	7 7 8 8 8	37 38 39 40 41	111 114 117 120 123
12-6 -12-8 12-10 13-5 13-11	7-11 8-1 8-4 8-5 8-7	78 81 85 89 93	26.4 25.2 24.0 26.3 28.9	6.29 6.37 6.44 6.73 7.03	18.07 21.45 26.23 21.23 18.39	8 8 8 9	42 43 44 45 46	126 129 132 135 138
14-1 14-3 14-10 15-4 15-6	8-9 8-11 9-1 9-3 9-5	97 101 105 109 113	27.6 26.3 28.9 31.6 30.2	7.09 7.16 7.47 7.78 7.83	21.18 24.80 21.19 18.90 21.31	9 9 9 9	47 48 49 50 51	141 144 147 150 153
15-8 15-10 16-5 16-7	9-7 9-10 9-11 10-1	118 122 126 131	28.8 27.4 30.1 28.7	7.89 7.96 8.27 8.33	24.29 28.18 24.24 27.73	10 10 10 10	52 53 54 55	156 159 162 165

**Figure 12.4.44** Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued

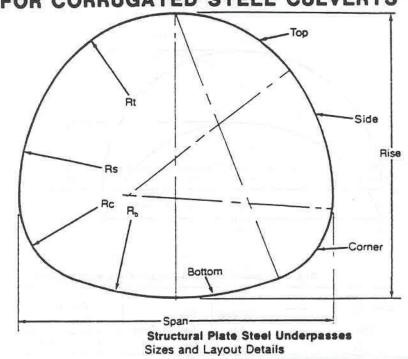


Sizes and Layout Details—Structural Plate Steel Pipe-Arches<sup>11</sup>  $6 \times 2$  in. Corrugations—Bolted Seams 31-In Corner Radius,  $R_c$ 

Dime	nsions		Lay	out Dimens	sions	P	eriphery	16
	D:	Waterway				No. of	Tot	tal
Span, ft-in.	Rise. ft-in.	Area, ft²	B in.	R <sub>t</sub> ft	R <sub>b</sub> ft	No. of Plates	N	Pi
13-3 13-6 14-0 14-2 14-5	9-4 9-6 9-8 9-10 10-0	97 102 105 109 114	38.5 37.7 39.6 38.8 37.9	6.68 6.78 7.03 7.13 7.22	16.05 18.33 16.49 18.55 21.38	8 8 8 8	46 47 48 49 50	138 141 144 147 150
14-11 15-4 15-7 15-10 16-3	10-2 10-4 10-6 10-8 10-10	118 123 127 132 137	39.8 41.8 40.9 40.0 42.1	7.48 7.76 7.84 7.93 8.21	18.98 17.38 19.34 21.72 19.67	9 9 10 10	51 52 53 54 55	153 156 159 162 163
16-6 17-0 17-2 17-5 17-11	11-0 11-2 11-4 11-6 11-8	142 146 151 157 161	41.1 43.3 42.3 41.3 43.5	8.29 8.58 8.65 8.73 9.02	21.93 20.08 22.23 24.83 22.55	10 10 10 10 10	56 57 58 59 60	168 17 17 17 17 18
18-1 18-7 18-9 19-3 19-6	11-10 12-0 12-2 12-4 12-6	167 172 177 182 188	42.4 44.7 43.6 45.9 44.8	9.09 9.38 9.46 9.75 9.83	24.98 22.88 25.19 23.22 25.43	10 10 10 10 11	61 62 63 64 65	18. 18. 18. 19.
19-8 19-11 20-5 20-7	12-8 12-10 13-0 13-2	194 200 205 211	43.7 42.5 44.9 43.7	9.90 9.98 10.27 10.33	28.04 31.19 28.18 31.13	11 11 11 12	66 67 68 69	190 20 20 20

Dimensions are to inside crests and are subject to manufacturing tolerances. N = 3 Pi = 0.6 in

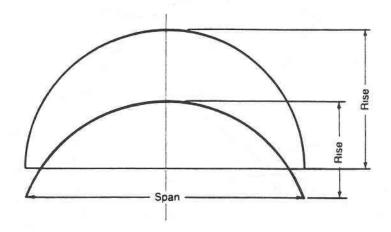
**Figure 12.4.44** Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued



			Peripher	у	Lay	out Dimens	ions in In	
	× Rise, nd in.	N	Pi	No. of Plates per Ring	Rt	R <sub>s</sub>	Rc	Rb
5-8 5-8 5-9 5-10 5-10	5-9 6-6 7-4 7-8 8-2	24 26 28 29 30	72 78 84 87 90	6 6 7 6	27 29 28 30 28	53 75 95 112 116	18 18 18 18	Flat Flat Flat Flat
12-2 12-11 13-2 13-10 14-1 14-6	11-0 11-2 11-10 12-2 12-10 13-5	47 49 51 53 55 57	141 147 153 159 165 171	8 9 11 11 11	68 74 73 77 77 78	93 92 102 106 115 131	38 38 38 38 38 38	136 148 161 168 183 174
14-10 15-6 15-8 16-4 16-5 16-9	14-0 14-4 15-0 15-5 16-0 16-3	59 61 63 65 67 68	177 183 189 195 201 204	11 12 12 12 12 12	79 83 82 86 88	136 139 151 156 159 168	38 38 38 38 38 38	193 201 212 217 271 246
17-3 18-4 19-1 19-6 20-4	17-0 16-11 17-2 17-7 17-9	70 72 74 76 78	210 216 222 228 234	12 12 13 13 13	90 99 105 107 114	174 157 156 158 155	47 47 47 47 47	214 248 262 293 310

All dimensions, to nearest whole number, are measured from inside crests. Tolerances should be allowed for specification purposes.  $6 \times 2$  in. Corrugations.

**Figure 12.4.44** Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued



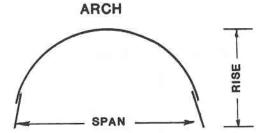
Representative Sizes of Structural Plate Steel Arches

Dimens	ions(1)				Non	ninal
Span,	Rise,	Waterway Area,	Rise	Radius.		ength
ft	ft-in.	ft²	Span(2)	in.	N(3)	Pi, in
6.0	1-9½	7½	0.30	41	9	27
	2-3½	10	0.38	37½	10	30
	3-2	15	0.53	36	12	36
7.0	2-4	12	0.34	45	11	33
	2-10	15	0.40	43	12	36
	3-8	20	0.52	42	14	42
8.0	2-11	17	0.37	51	13	39
	3-4	20	0.42	48½	14	42
	4-2	26	0.52	48	16	48
9.0	2-11	18½	0.32	59	14	42
	3-101/2	26½	0.43	55	16	48
	4-81/2	33	0.52	54	18	54
10.0	3-5½	25	0.35	64	16	48
	4-5	34	0.44	60½	18	54
	5-3	41	0.52	60	20	60
11.0	3-6	27½	0.32	73	17	51
	4-5½	37	0.41	67½	19	57
	5-9	50	0.52	66	22	66
12.0	4-0½	35	0.34	77½	19	57
	5-0	45	0.42	73	21	63
	6-3	59	0.52	72	24	72
13.0	4-1	38	0.32	86½	20	60
	5-1	49	0.39	80½	22	66
	6-9	70	0.52	78	26	78
14.0	4-7½	47	0.33	91	22	66
	5-7	58	0.40	86	24	72
	7-3	80	0.52	84	28	84

(Table continued on following page)

(¹)Dimensions are to inside crests and are subject to manufacturing tolerances. (³)R/S ratio varies from 0.30 to 0.52. Intermediate spans and rises are available. (³)W = 3 Pi = 9.6 in.  $6 \times 2$  in. Corrugations—Bolted Seams.

Figure 12.4.44 Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued

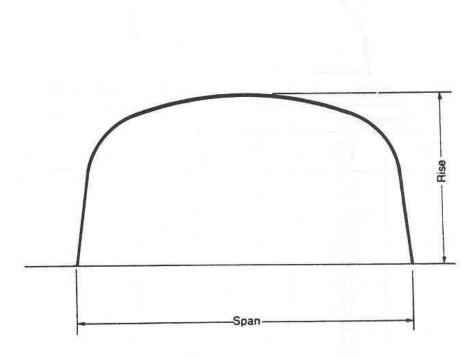


Continued. Representative Sizes of Structural Plate Steel Arches

Dime	ensions(1)	Waterway	Rise			ninal ength
Span, ft	Rise. ft-in.	Area, ft²	over Span(2)	Radius, in.	N(3)	Pi, in
15.0	4-7½	50	0.31	101	23	69
	5-8	62	0.38	93	25	75
	6-7	75	0.44	91	27	81
	7-9	92	0.52	90	30	90
16.0	5-2	60	0.32	105	25	75
	7-1	86	0.45	97	29	87
	8-3	105	0.52	96	32	96
17.0	5-2½	63	0.31	115	26	78
	7-2	92	0.42	103	30	90
	8-10	119	0.52	102	34	102
18.0	5-9	75	0.32	119	28	84
	7-8	104	0.43	109	32	96
	8-11	126	0.50	108	35	105
19.0	6-4	87	0.33	123	30	90
	8-2	118	0.43	115	34	102
	9-51/2	140	0.50	114	37	111
20.0	6-4	91	0.32	133	31	93
	8-3½	124	0.42	122	35	105
	10-0	157	0.50	120	39	117
21.0	6-11	104	0.33	137	33	99
	8-10	140	0.42	128	37	111
	10-6	172	0.50	126	41	123
22.0	6-11	109	0.31	146	34	102
	8-11	146	0.40	135	38	114
	11-0	190	0.50	132	43	129
23.0	8-0	134	0.35	147	37	111
	9-10	171	0.43	140	41	123
	11-6	208	0.50	138	45	135
24.0	8-6	150	0.35	152	39	117
	10-4	188	0.43	146	43	129
	12-0	226	0.50	144	47	141
25.0	8-6½	155	0.34	160	40	120
	10-10½	207	0.43	152	45	135
	12-6	247	0.50	150	49	147

(1)Dimensions are to inside crests and are subject to manufacturing tolerances. (2)R/S ratio varies from 0.30 to 0.52. Intermediate spans and rises are available. (3)W = 3 Pi = 9.6 in.  $6 \times 2$  in. Corrugations—Bolted Seams.

**Figure 12.4.44** Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued



Layout Details Corrugated Steel Box Culverts

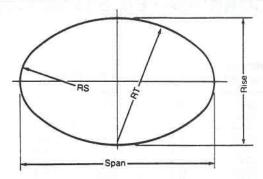
Rise. ft-in.	Span. ft-in.	Area ft²	Rise. ft-in	Span. ft-in.	Area ft²
2-7 2-8	9-8	20.8	3-9	12-10	41.0
2-8	10-5	23.2	3-10	13-6	44.5
2-9	11-1	25.7	3-10	17-4	55.0
2-10	11-10	28.3	3-11	14-2	48.2
2-11	12-6	31.1	3-11	18-0	59.1
3-1	13-3	34.0	4-1	14-10	52.0
3-2	13-11	37.1	4-1	18-8	63.4
3-2 3-3	14-7	40.4	4-1 4-2 4-2	10-7	36.4
3-4	10-1	28.4	4-2	15-6	55.9
3-4 3-5	10-10	31.4	4-3	11-2	39.9
3-5	15-3	43.8	4-3	19-4	67.9
3-6	11-6	34.5	4-4	11-10	43.5
3-5 3-6 3-6	16-0	47.3	4-4	16-2	60.1
3-8	12-2	37.7	4-4 4-5	12-6	47.3
3-8 3-8	16-8	51.1	4-6	13-2	51.2

**Figure 12.4.44** Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued

#### Continued. Layout Details Corrugated Steel Box Culverts

Rise,	Span,	Area	Rise,	Span,	Area
ft-in.	ft-in.	ft²	ft-in.	ft-in.	ft <sup>2</sup>
4-6	16-10	64.4	6-9	13-7	77.9
4-7	17-6	68.9	6-9	16-9	99.3
4-7	20-8	77.6	6-10	14-2	83.3
4-8	13-10	55.3	6-10	17-4	105.1
4-9	14-6	59.5	7-0	14-9	88.9
4-9	18-1	73.5	7-0	17-11	111.1
4-10	15-1	63.8	7-0	20-8	127.2
4-11	11-0	44.7	7-1	15-4	94.6
4-11	18-9	78.4	7-2	18-6	117.3
5-0	11-7	48.7	7-3	12-3	71.5
5-0	15-9	68.3	7-3	15-10	100.5
5-1	12-3	52.9	7-4	12-10	77.1
5-1	16-4	73.0	7-4	16-5	106.5
5-1	19-5	83.4	7-4	19-1	123.6
5-2	12-10	57.2	7-5	13-5	82.8
5-3	17-0	77.8	7-6	13-11	88.6
5-4	13-6	61.7	7-6	17-0	112.7
5-5	14-1	66.2	7-8	14-6	94.5
5-5	17-7	82.8	7-8	17-6	119.0
5-5	20-8	94.1	7-9	15-0	100.6
5-6	14-9	71.0	7-9	18-1	125.5
5-7	18-3	88.0	7-11	15-7	106.8
5-8	11-5	53.3	7-11	18-7	132.1
5-8	15-4	75.8	8-0	12-8	81.1
5-8	18-10	93.4	8-0	16-1	113.1
5-9	12-0	57.9	8-1	19-2	138.9
5-9	16-0	80.9	8-2	16-8	119.6
5-10	12-7	62.6	8-2	13-9	93.3
5-10	19-6	98.9	8-3	19-8	145.9
5-11	16-7	86.1	8-4	17-2	126.2
6-0	13-3	67.4	8-5	14-10	106.0
6-1	13-10	72.4	8-5	17-8	133.0
6-1	17-2	91.4	8-7	18-3	139.9
6-2	14-5	77.5	8-7	20-9	160.3
6-2	17-9	96.9	8-8	15-10	119.2
6-2	20-8	110.6	8-9	18-9	147.0
6-4	15-0	82.7	8-11	16-10	132.9
6-4	18-4	102.6	8-11	19-3	154.2
6-5	11-10	62.2	9-1	19-9	161.6
6-5	15-7	88.1	9-3	17-10	147.1
6-6 6-7 6-7 6-8 6-8	18-11 12-5 16-2 13-0 19-6	108.5 67.3 93.6 72.5 114.5	9-5 9-6 9-10 10-2	20-9 18-10 19-10 20-9	176.9 162.0 177.4 193.5

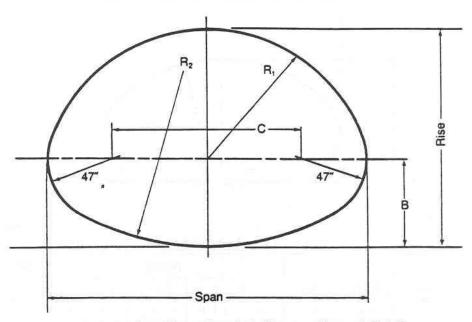
Figure 12.4.44 Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued



Long Span Horizontal Ellipse Sizes and Layout Details

		ing spe	an no	112OIII			31200	and	Layout De	
					Perip	hery			Inside	Radius
Span,	Rise,	Area,	Top Bot		Sic	de	Tot	al	Тор	Side
ft-in.	ft-in.	ft²	N	Pi	N	Pi	N	Pi	Rad. in.	Rad. in
19- 4 20- 1 20- 2 20-10 21- 0	12- 9 13- 0 11-11 12- 2 15- 2	191 202 183 194 248	22 23 24 25 23	66 69 72 75 69	10 10 8 8 13	30 30 24 24 24 39	64 66 64 66 72	192 198 192 198 216	12- 6 13- 1 13- 8 14- 3 13- 1	4- 6 4- 6 3- 7 3- 7 5-11
21-11 22- 6 23- 0 23- 3 24- 4	13- 1 15- 8 14- 1 15-11 16-11	221 274 249 288 320	26 25 27 26 27	78 75 81 78 81	9 13 10 13 14	27 39 30 39 42	70 76 74 78 82	210 228 222 234 246	14-10 14-3 15-5 14-10 15-5	4- 1 5-11 4- 6 5-11 6- 4
24- 6 25- 2 25- 5 26- 1 26- 3	14- 8 14-11 16- 9 18- 2 15-10	274 287 330 369 320	29 30 29 29 31	87 90 87 87 93	10 10 13 15 11	30 30 39 45 33	78 80 84 88 84	234 240 252 264 252	16- 6 17- 1 16- 6 16- 6 17- 8	4- 6 4- 6 5-11 6-10 4-11
27- 0 27- 2 27-11 28- 1 28-10	16- 2 19- 1 19- 5 17- 1 17- 5	334 405 421 369 384	32 30 31 33 34	96 90 92 99 102	11 16 16 12 12	33 48 48 36 36	96 92 94 90 92	258 276 282 270 276	18- 3 17- 1 17- 8 18-10 19- 5	4-11 7- 3 7- 3 5- 5
29- 5 30- 1 30- 3 31- 2 31- 4	19-11 20- 2 17-11 21- 2 18-11	455 472 415 512 454	33 34 36 35 37	99 102 108 105 111	16 16 12 17 13	48 48 36 51 39	98 100 96 104 100	294 300 288 312 300	18-10 19- 5 20- 7 20- 0 21- 1	7- 3 7- 3 5- 5 7- 9 5-11
32- 1 32- 3 33- 0 33- 2 34- 1	19- 2 22- 2 22- 5 20- 1 23- 4	471 555 574 512 619	38 36 37 39 38	114 108 111 117 114	13 18 18 14 19	39 54 54 42 57	102 108 110 106 114	306 324 330 318 342	21- 8 20- 7 21- 1 22- 3 21- 8	5-11 8- 2 8- 2 6- 4 8- 8
34- 7 34-11 35- 1 35- 9 36- 0	20- 8 21- 4 24- 4 25- 9 22- 4	548 574 665 718 619	41 41 39 39 42	123 123 117 117 126	14 15 20 22 16	42 45 60 66 48	110 112 118 122 116	330 336 354 366 348	23- 5 23- 5 22- 3 22- 3 24- 0	6- 4 6-10 9- 1 10- 0 7- 3
36-11 37- 2 38- 0 38- 8 40- 0	25- 7 22- 2 26- 7 27-11 29- 7	735 631 785 843 927	41 44 44 42 43	123 132 132 126 129	21 15 22 24 26	63 45 66 72 78	124 118 128 132 138	372 354 384 396 414	23- 5 25- 2 24- 0 24- 0 27-11	9- 7 6-10 10- 0 10-11

**Figure 12.4.44** Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued

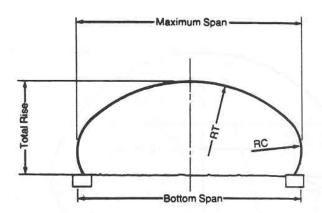


Long Span Pipe Arch Sizes and Layout Details

						Peri	phery					Inside	Radius
Sana	Dica	Area.	Total No.	Ţ	ор	Bot	tom	To	tal	В,	C,	R <sub>1</sub> ,	R <sub>2</sub> ,
Span, ft-in.	Rise, ft-in.	ft²	Plates	N	N Pi		N Pi		N Pi		in.	in.	in.
20- 0 20- 6 21- 5 21-11 22- 5	13-11 14- 3 14- 6 14-11 15- 3	218 231 243 256 270	10 10 11 11 11	34 36 36 38 40	102 108 108 114 120	20 20 22 22 22 22	60 60 66 66 66	68 70 72 74 76	204 210 216 222 228	62.8 61.4 65.3 63.7 62.1	146.2 152.3 162.8 168.9 174.6	122.5 124.7 131.4 133.5 135.5	223.6 255.7 236.7 268.1 307.1
23- 4 24- 2 24- 8 25- 2 25- 7	15- 7 15-11 16- 2 16- 7 16-11	284 297 312 326 342	11 12 12 12 12	40 40 42 44 46	120 120 126 132 138	24 26 26 26 26	72 78 78 78 78 78	78 80 82 84 86	234 240 246 252 258	66.2 70.7 68.8 66.9 64.8	185.5 196.2 202.2 207.9 213.3	142.4 149.7 151.4 153.2 155.0	280.2 262.1 292.2 328.6 373.3
26- 7 27- 6 28- 0 28- 5 29- 4	17- 3 17- 6 17-10 18- 3 18- 6	357 372 388 405 421	12 12 12 13 13	46 46 48 50 50	138 138 144 150 150	28 30 30 30 30 32	84 90 90 90 96	88 90 92 94 96	264 270 276 282 288	69.4 74.2 72.1 69.9 74.8	224.7 235.8 241.5 246.8 258.2	162.1 169.6 171.1 172.7 180.2	339.4 315.8 350.2 392.3 361.1
30- 4	18-10	438	14	52	156	34	102	100	300	80.0	269.4	188.2	339.

Machidae 14M for two M7 corner plates

Figure 12.4.44 Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued

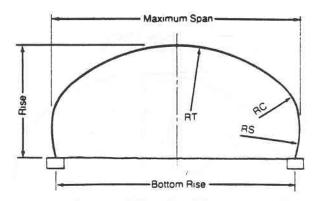


Long Span Low Profile Arch Sizes and Layout Details

						Peri	phery			Inside	Radius
Max. Span,	Bottom Span,	Total Rise,	Area,	1	op q	Si	de	To	otal	Тор	Side
ft-in.	ft-in.	ft-in.	ft²	N	Pi	N	Pi	N	Pi	rad. in.	rad. in.
20- 1 19- 5 21- 6 22- 3 23- 0	19-10 19- 1 21- 4 22- 1 22- 9	7- 6 6-10 7- 9 7-11 8- 0	121 105 134 140 147	23 23 25 26 27	69 69 75 78 81	6566	18 15 18 18 18	35 33 37 38 39	105 99 111 114 117	13- 1 13- 1 14- 3 14-10 15- 5	4- 6 3- 7 4- 6 4- 6 4- 6
23- 9 24- 6 25- 2 25-11 27- 3	23- 6 24- 3 25- 0 25- 9 27- 1	8- 2 8- 4 8- 5 8- 7 10- 0	154 161 169 176 217	28 29 30 31 31	84 87 90 93 93	66668	18 18 18 18 24	40 41 42 43 47	120 123 126 129 141	16- 0 16- 6 17- 1 17- 8 17- 8	4- 6 4- 6 4- 6 4- 6 6- 4
28- 1 28- 9 28-10 30- 3 30-11	27-11 28- 7 28- 8 30- 1 30- 9	9- 7 10- 3 9- 8 9-11 10- 8	212 234 221 238 261	33 33 34 36 36	99 99 102 108 108	7 8 7 7 8	21 24 21 21 21 24	47 49 48 50 52	141 147 144 150 156	18-10 18-10 19- 5 20- 7 20- 7	5- 5 6- 4 5- 5 5- 5 6- 4
31- 7 31- 0 32- 4 31- 9 33- 1	31- 2 30-10 31-11 31- 7 32- 7	12- 1 10- 1 12- 3 10- 3 12- 5	309 246 320 255 330	36 37 37 38 38	108 111 111 114 114	10 7 10 7 10	30 21 30 21 30	56 51 57 52 58	168 153 171 156 174	20- 7 21- 1 21- 1 21- 8 21- 8	7- 3 5- 5 7- 3 5- 5 7- 3
33- 2 34- 5 34- 7 37-11 35- 4	33- 0 34- 1 34- 6 37- 7 35- 2	11- 1 13- 3 11- 4 15- 8 11- 5	289 377 308 477 318	39 39 41 41 42	117 117 123 123 126	8 11 8 14 8	24 33 24 42 24	55 61 57 69 58	165 183 183 207 174	22- 3 22- 3 23- 5 23- 5 24- 0	6- 4 8- 2 6- 4 10-11 6- 4
38- 8	38- 4	15- 9	490	42	126	14	42	70	210	24- 0	10-11

NOTE: Larger sizes available for special designs.

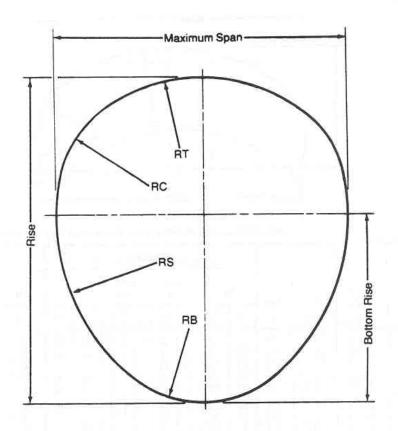
Figure 12.4.44 Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued



Long Span High Profile Arch Sizes and Layout Details

							Peri	hery				Ins	ide Radius	
Max. Span,	Bottom Span,	Total Rise,	Area	To	op.		per de	Lov Si	wer de	To	ital	Top Radius,	Upper Side.	Lower
ft-in.	ft-in.	ft-in.	ft²	N	Pi	N	Pi	N	Pi	N	Pi	ft-in.	ft-in.	Side, ft-in.
20- 1 20- 8 21- 6 22-10 22- 3	19- 6 18-10 19-10 19-10 20- 7	9- 1 12- 1 11- 8 14- 7 11-10	152 214 215 285 225	23 23 25 25 25 26	69 69 75 75 78	5 6 5 7 5	15 18 15 21 15	36686	9 18 18 24 18	39 47 47 55 48	117 141 141 165 144	13- 1 13- 1 14- 3 14- 3 14-10	4- 6 5- 5 4- 6 6- 4 4- 6	13- 1 13- 1 14- 1 14- 1
22-11 23- 0 24- 4 23- 9 24- 6	20- 0 21- 5 21- 6 22- 2 21-11	14- 0 12- 0 14-10 12- 1 13- 9	276 235 310 245 289	26 27 27 28 29	78 81 81 84 87	6 5 7 5 5	18 15 21 15 15	8 6 8 6 8	24 18 24 18 24	54 49 57 50 55	162 147 171 150 165	14-10 15- 5 15- 5 16- 0 16- 6	5- 5 4- 6 6- 4 4- 6 4- 6	14-10 15- 15- 16- 16-
25- 9 25- 2 26- 6 25-11 27- 3	23- 2 23- 3 24- 0 24- 1 24-10	15- 2 13- 2 15- 3 13- 3 15- 5	335 283 348 295 360	29 30 30 31 31	87 90 90 93 93	7 5 7 5 7	21 15 21 15 21	8 7 8 7 8	24 21 24 21 24	59 54 60 55 61	177 162 180 165 183	16- 6 17- 1 17- 1 17- 8 17- 8	6- 4 4- 6 6- 4 4- 6	16- 17- 17- 17- 17-
27- 5 29- 5 28- 2 30- 1 30- 3	25- 8 27- 1 25-11 26- 9 28- 2	13- 7 16- 5 14- 5 18- 1 15- 5	317 412 349 467 399	33 33 34 34 36	99 99 102 102 108	58586	15 28 15 24 18	7 8 8 10 8	21 24 24 30 24	57 65 60 70 64	171 195 180 210 192	18-10 18-10 19-5 19-5 20-7	4- 6 7- 3 4- 6 7- 3 5- 5	18-1 18-1 19- 19- 20-
31- 7 31- 0 31- 8 32- 4 31- 9	28- 4 29- 0 28- 6 27-11 28- 8	18- 4 15- 7 17- 9 19-11 17- 3	497 413 484 554 470	36 37 37 37 37 38	108 111 111 111 114	8 6 7 8 6	24 18 21 24 18	10 8 10 12 10	30 24 30 36 30	72 65 71 77 70	216 195 213 231 210	20- 7 21- 1 21- 1 21- 1 21- 8	7- 3 5- 5 6- 4 7- 3 5- 5	20- 21- 21- 21- 21- 21-
33- 1 32- 6 33-10 34- 0 34- 7	28- 9 29- 6 29- 7 31- 2 30- 7	20- 1 17- 4 20- 3 17- 8 19-10	571 484 588 514 591	38 39 39 41 41	114 117 117 123 123	8 6 8 6 7	24 18 24 18 21	12 10 12 10 12	36 30 36 30 36	78 71 79 73 79	234 213 237 219 237	21- 8 22- 3 22- 3 23- 5 23- 5	7- 3 5- 5 7- 3 5- 5 6- 4	21- 22- 22- 23- 23-
35- 3 37- 3 34- 8 35- 4 36- 0	30- 7 32- 6 31-11 31- 5 31- 5	21- 3 23- 5 17-10 20- 0 21- 5	645 747 529 608 663	41 41 42 42 42 42	123 123 126 126 126 126	8 11 6 7 8	24 33 18 21 24	13 13 10 12 13	39 39 30 36 39	83 89 74 80 84	249 267 222 240 252	23- 5 23- 5 24- 0 24- 0 24- 0	7- 3 10- 0 5- 5 6- 4 7- 3	23- 23- 24- 24- 24-
38- 0	33- 5	23- 6	767	42	126	11	33	13	39	90	270	24- 0	10- 0	24-

**Figure 12.4.44** Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued



Long Span Pear Shape Sizes and Layout Details

								Peri	phery					Inside Radius				
		Тор		Corner		Side		Bot	tom	Total		Bottom Radius,	Side Radius,	Corner Radius,	Top Radius,			
	Area	N	Pi	N	Pi	N	Pi	N	Pi	N	Pi	ft-in.	ft-in.	ft-in.	ft-in.			
23- 8 24- 0 25- 6 24-10 27- 5	25- 8 25-10 25-11 27- 8 27- 0	14-11 15- 1 15-10 16- 9 18- 1	481 496 521 544 578	25 22 27 27 27 30	75 66 81 81 90	5 7 7 5 6	15 21 21 15 18	24 22 20 25 26	72 66 60 75 78	15 20 21 18 16	30 60 63 54 48	98 100 102 105 110	294 300 306 315 330	8-11 9-11 10- 7 9- 3 9- 7	16- 7 17- 4 18- 1 19- 8 20- 4	6-3 7-0 6-11 5-9 4-7	14- 8 16- 2 15-10 15-11 19-11	
26- 8 28- 1 28- 7 30- 0 30- 0	28- 3 27-10 30- 7 29- 8 31- 2	18- 0 16-10 19- 7 20- 0 19-11	593 624 689 699 736	28 27 32 32 34	84 81 96 96 102	5 8 7 8 7	15 24 21 24 24 21	30 22 24 23 24	90 66 72 69 72	12 25 24 25 26	36 75 72 75 78	110 112 118 119 122	330 336 354 357 366	8- 0 12- 2 11- 2 11-11 12- 1	20- 1 19- 0 24- 0 24- 0 24- 0	4- 9 7- 3 7- 0 6- 7 7- 0	20-11 20- 5 18- 2 21-10 19- 3	

**Figure 12.4.44** Standard Sizes for Corrugated Steel Culverts (Source: American Iron and Steel Institute), continued

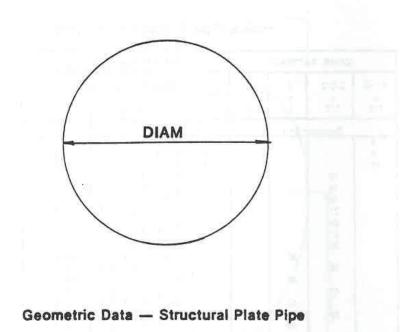
Helical Pipe Availability, Weights

CORR. PA	ATTERN			WEIG	SHT (Lb	s/Lines	I Ft.)	
1-1/2 2-2/3	3	6.		Equi	v. Stan	dard Ga	uge	
1/4 1/2	x 1	1	18	16	14	12	10	8
Diamete	er (In.)			14.0				
6 8 10 12 15 18 21 24 27 30 36 42 48 54 60 66 72 78	30 36 42 48 54 60 66 72 78 84 90 96 102 108 114 120	48 54 60 72 78 84 90 96 102 108 114	1.4 1.8 2.2	1.7 2.2 2.7 3.9 4.7 5.4 6.2 7.0 7.8 8.9 10.7 12.4 14.1 12.5 15.8 14.0 17.6 15.5 17.0	4.0 4.9 5.9 6.8 7.8 8.7 9.6 11.2 11.5 13.4 15.5 17.7 15.6 19.9 17.5 22.0 19.4 21.3 26.3 23.2 28.5 25.1 30.7	5.5 6.8 8.1 9.4 10.7 12.1 13.4 15.5 16.0 18.5 21.2 24.5 21.8 27.5 24.5 27.2 24.5 27.2 29.8 36.5 32.5 32.5 35.2 42.5 35.4 40.5 48.4 48.5 57.4 57.4 57.4 57.4 57.4 57.4 57.4 57	13.8 15.4 17.1 19.9 20.5 23.7 23.8 27.5 27.2 23.4 28.1 30.5 35.2 31.9 39.0 34.9 37.2 38.4 46.7 41.8 50.5 45.2 54.3 48.7 58.2 56.8 69.7 62.4 73.5 65.8 77.3	32.7 34.1 36.7 42.4 46.6 48.8 55.2 55.0 65.1 70.0 63.3 75.8 80.0 93.0

NOTES: 1. Sizes 6" thru 10" are available in helical corrugation only.

2. Sizes 12" through 21" in helical configuration have corrugation depth of 7/16" rather than 1/2".

**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association)

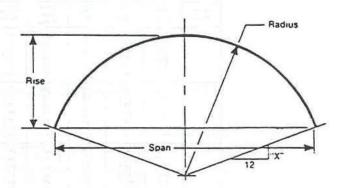


Nom. Diam. In.	Area Sq. Ft.	Total N	Nom. Diam. In.	Area Sq. Ft.	Total N
60	19	20	162	145	54
66	23	22	168	156	56
72	27	24	174	167	58
78	32	26	180	179	60
84	38	28	186	191	62
90	44	30	192	204	64
96	50	32	198	217	66
102	56	34	204	231	68
108	63	36	210	245	70
114	71	38	216	259	72
120	79	40	222	274	74
126	87	42	228	289	76
132 138 144 150	95 104 114 124 134	44 46 48 50 52	234 240 246 252	305 321 337 354	78 80 82 84

**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued

### GEOMETRIC DATA - ARCH

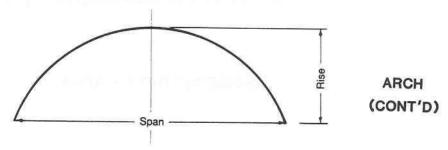
	"X" Values Fo	r Rise/Span Ratio	
R/S Ratio	-x-	R/S Ratio	-X-
.30	6.40	.42	2.10
.31	5.96	.43	1.82
.32	5.54	.44	1.54
.33	5.13	.45	1,27
.34	4.74	.46	1,00
.35	4.37	.47	,74
.36	4.01	.48	.48
.37	3.67	.49	.24
.38	3.33	.50	.00
.39 .40	3.01 2.70 2.40	.51 .52	.24



**Typical Section** 

Span Ft.In.	Rise Fl.In.	Area Sq.Ft.	Total N	Rise/ Span Ratio	Radius Inches	Span Ft.In.	Rise Fl.In.	Area Sq.Fl.	Total N	Rise/ Span Ratio	Radius
5-0	2-7 2-3 1-9	10.4 8.5 6.5	10 9 8	.52 .44 .36	30 30¼ 31¼	9-0	4-8 4-3 3-10 3-5	33.4 29.9 26.3 22.8	18 17 16 15	.50 .48 .43	54 54 54: <sub>2</sub> 56
6-0	3-2 2-9	14.9	12	.52	36 36¼		2-11	19.1	14	.33	59
	2-4	10.2	10	.38	37½ 40½	10-0	5-2 4-10 4-5	41 2 37.3 33.3	20 19 18	.52 .48 .44	60 60 60'5
7-0	3-8 3-3 2-10	20.3 17.5 14.8	14 13 12	.52 .46 .40	42 42 43		3-11 3-6 3-0	29.4 25.3 21.1	17 16 15	.40 .35 .30	61'-7 64 68'-7
	2-4	12.0	11	.34	45'.	11-0	5-8	49.8	22	.52	66
8-0	4-2 3-9 3-4	26.4 23:3 20.2	16 15 14	.52 .47 .42	48 48 48,		5-4 4-11 4-6 4-0	45.5 41.2 36.8	21 20 19	.48 .45 .41	66 66' <sub>2</sub> 67' <sub>2</sub> 69' <sub>4</sub>
	2-11 2-5	17.0 13.6	13	.36 .30	5012		3-6	32.4 27.8	18	.36 .32	721.

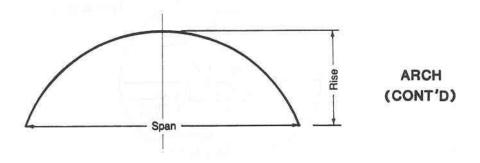
**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued



#### Geometric Data-Arch (Continued)

Span FLIn.	Rise FLIn.	Area Sq.FL	Total N	Rise/ Span Ratio	Radius Inches	Span FLIn	Rise Ft.In.	Area Sq.FL	Total N	Rise/ Span Ratio	Radiu
12-0	6-3 5-10 5-5 5-0 4-7 4-1	59.3 54.5 49.8 45.0 40.2 35.3	24 23 22 21 20 19	.52 .49 .45 .42 .38 .34	72 72 72'7 73'4 75 77'7	20-0 cont.	9-2 8-9 8-3 7-10 7-4 5-10 6-4	140.4 132.4 124.4 116.3 108.4 99.8 91.2	37 36 35 34 33 32 31	.46 .44 .41 .39 .37	1201/4 121 1221/4 1231/4 1252/4
13-0	6-9 6-4 5-11 5-6 5-1 4-7 4-1	69.5 64.4 59.3 54.1 48.9 43.6 38.1	25 24 23 22 21 20	.52 .49 .45 .42 .39 .35	78 78 / 78 / 78 / 79 80 / 4 82 / 4 86 / 4	21-0	10-10 10-6 10-1 9-8 9-3 8-10	181.0 172.7 164.3 156.0 147.6 139.2	42 41 40 39 38 37	52 50 48 .46 .44	1321/ <sub>5</sub> 126 126 126 126 1261/ <sub>5</sub> 127 128
14-0	7-3 6-10 6-5 6-0 5-7 5-2	60.6 75.1 69.5 64.0 58.4	28 27 26 25 24	52 49 46 43 40	84 84 84'' <sub>4</sub> 85 86		8-4 7-11 7-5 6-11 6-4	130.7 122.2 113.5 104.6 95.4	36 35 34 33 32	.40 .38 .35 .33 .30	129% 131% 133% 137% 142
032020	4-8	52.7 46.9	23 22	.37	88 91%	22-0	11-5 11-0 10-7	198.6 189.9 181.1	43	.52 .50 .48	132 132 132
15-0	7-9 7-5 7-0 6-7 6-1 5-8 5-2 4-8	92.5 86.5 80.6 74.7 68.7 62.6 56.4 50.0	30 29 28 27 25 25 24 23	.52 .49 .46 .44 .41 .38 .34 .31	90 90 % 91 92 93 % 96 % 100 %		10-2 9-9 9-4 8-11 8-5 7-11 7-5 6-11	172.4 163.6 154.8 146.0 137.0 127.9 118.7 109.2	41 40 39 38 37 36 35 35	45 44 42 40 38 36 34	132% 133 1337, 135 1357, 139 142%
16-0	8-3 7-11 7-6 7-1 6-8 6-2 5-9 5-3	105.2 98.9 92.5 86.2 79.8 73.3 66.8 60.0	32 31 30 29 28 27 26 25	.52 .49 .47 .44 .41 .39 .36	96 96 964, 964, 974, 994, 1014, 105	23-0	11-11 11-6 11-1 10-6 10-3 9-10 9-5 5-11	217.1 207.9 198.8 189.5 180.5 171.3 162.0 152,7	45 45 43 42 41 49 39	.52 .50 .48 .47 .45 .43 .41	138 138 138 1387, 139 1397, 1407,
17-0	8-10 8-5 8-0 7-7	118.7 112.0 105.2 96.5	34 33 32	.52 .49 .47	102 102 1021/4		8-6 8-0 7-6 6-11	143.2 133.6 123.6 113.6	38 37 36 35	.37 .35 .33	14414 14714 151 158
	7-2 6-9 6-3 5-9 5-3	91.7 84.9 77.9 70.9 63.5	31 30 29 28 27 26	.45 .42 .39 .37 .34	102½ 103½ 105 107 110 114½	24-0	12-5 12-0 11-7 11-3 10-10	236.3 226.8 217.2 207.7 198.1	48 47 46 45	.52 .50 .48 .47 .45	144 144 144 144 <sup>1</sup> , 144 <sup>1</sup> ,
18-0	9-4 8-11 8-6 8-1 7-8 7-3 6-9	133.1 125.9 116.8 111.6 104.5 97.2 69.9	36 35 34 33 32 31 30	.52 .50 .47 .45 .43 .40	108 108 108½ 108½ 109½ 110½ 112½		10-4 9-11 9-6 9-0 8-6 8-0 7-6	188.5 178.9 169.2 159.3 149.4 139.2 128.9	43 42 41 40 39 38 37	41 39 38 36 33 31	1451-2 1461-7 148 150 1521/4 1554-1
	5-4 5-9	82,5 74.8	29 28	.35 .32	115 1182,	25-0	12-11	256.4 246.4	50 49	.52 .50	150 150
19-0	9-10 9-5 9-0 8-8 8-2 7-9 7-4 6-10 6-4 5-10	148.2, 140.7 133.2 125.6 115.0 110.4 102.7 94.9 86.9 78.7	35 37 36 35 34 33 32 31 30 29	52 50 45 45 41 38 36 31	1182, 114 114 1144, 1144, 1154, 1167, 118 1207, 1237, 1287,		12-2 11-9 11-4 10-11 10-5 10-0 9-6 9-1 8-7 8-1 7-6	236.5 226.6 216.6 206.6 196.6 186.4 176.3 185.9 155.4 144.7 133.7	48 47 45 45 44 43 42 41 40 39 38	.49 .47 .45 .44 .42 .40 .38 .36 .34 .32	150% 150% 150% 151% 152% 153% 155% 157% 160% 164%
20-0	10-4 10-0 9-7	164.2 156.3 146.3	40 39 38	.52 .50 48	120 120 120	26-0	13-5 13-1 12-8	277 3 266.9 256.6	52 51 50	52 50 49	156 156 156

**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued

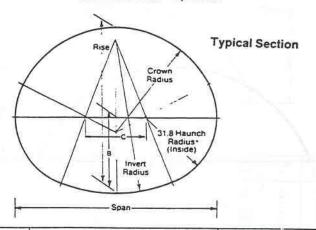


#### Geometric Data-Arch (Continued)

Span Ft.in.	Rise Ft.In.	Area Sq.Ft.	Total N	Rise/ Span Ratio	Radius Inches	Span Ft.In.	Rise FLIn.	Area Sq.Ft.	Total N	Rise/ Span Ratio	Radius
26-0	12-3	246.2	49	47	1561/4	28-0	10-2	208.8	46	.36	176%
cont.	11-10	235.9	48	46	1563/4	cont.	9-8	197.1	45	.35	1791/2
	11-5	225.5	47	44	1571/4		9-2	185.1	44	33	1831/4
	11-0	215.1	46	.42	1581,	Į	8-8	172.9	43	.31	188
	10-6	204.6	45	40	1591/2						-
	10-1	194.0	44	.39	161	29-0	15-0	344.8	58	.52	174
	9-7	183.3	43	.37	1631/4		14-7	333.3	57	.50	174
	9-1	172.4	42	.35	166		14-2	321.7	56	.49	174
	8-7	161.4	41	33	1691/2		13-10	310.2	55	.48	1741/4
	8-1	150.1	40	.31	174		13-5	298.6	54	.46	1741/2
							13-0	287.1	53	.45	175
27-0	14-0	299.0	54	52	162		12-6	275.4	52	43	175%
	13-7	288.2	53	.50	162		12-1	263.8	51	42	176%
	13-2	277.5	52	49	162		11-8	252.0	50	40	1781/4
	12-9	266.7	51	47	16.214		11-2	240.2	49	.39	180
	12-4	256.0	50	46	162%		10-9	228.2	48	.37	182
	13-11	245.2	49	44	1631/4		10-3	216.1	47	.35	1843/
	11-6	234.4	48	43	164		9-9	203.8	46	34	188
	11-1	223.5	47	41	1651/4		9-2	191.3	45	32	1921/4
	10-7	212.6	46	39	1663		8-8	178.5	44	30	1977/
	10-2	201.4	45	38	1683		0.0	176.5	77	.30	13174
	9-8	190.2	44	36	1711	30-0	15-6	369.0	60	.52	180
	9-2	178.8	43	34	1741	30.0	15-1	357.1	59	50	180
	8-7	167.2	42	.32	1781/2		14-9	345.1	58	49	180
	8-1	155.3	41	.30	1832		14-4	333.2	57	48	180
		100.0		.50	100-1		13-11	321.2	56	46	180%
28-0	14-6	321.5	56	.52	168		13-6	309.2	55	45	181
	14-1	310.4	55	50	168		13-1	297.2	54	44	1813/
	13-8	299.2	54	49	168		12-7		53	42	1827
	13-3	288.1	53	47	1681		12-2	285.1	52	41	184
	12-10	276.9	52	46	1681		11-9	273.0	51	39	1851/2
	12-5	265.7	51	44	1691		11-3	260.8	50	.37	
	12-0	254.5	50	43	170		10-9	248.5	49		18717
	11-7		49	41	171			236.0		.36	190
	11-1	243.2					10-3	223.3	48	34	193
	10-8	231.9	48 47	40	172'2		9-9	210.5	47	32	197
	10-8	220.4	-/	38	17414		9-2	197.3	46	31	2013/4

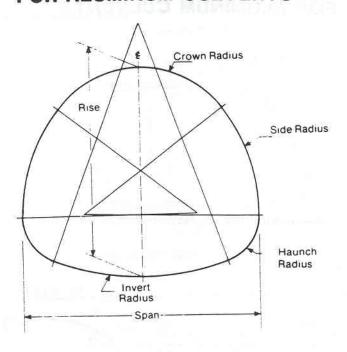
**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued





Span	Rise	Area		Requ	red N		Inside	Radius	-	C
FtIn.	FtIn.	Sq. Ft.	Total	Crown	Invert	Haunch	Crown In.	invert in.	В	-
6-7 6-11	5-8 5-9	29.6 31.9	25 26	8 9	3	7	41.5	69.9 102.9	32.5 32.4	15.3
7-3	5-11	34.3	27	10	3	7	45.6	188.3	32.2	23.8
7-9	6-0	36.8	28	9	5	7	51.6	83.8	33.8	29.0
8-1	6-1	39.3	29	10	5	7	53.3	108.1	33.5	33.3
8-5	6-3	41.9	30	11	5	7	54.9	150.1	33.2	37.4
8-10	6-4	44.5	31	10	7	7	63.3	93.0	35.6	42.8
9-3	5-5	47.1	32	11	7	7	64.4	112.6	35.2	47.1
9-7	6-6	49.9	33	12	7	7	65.4	141.6	34.7	51.3
9-11	6+8	52.7	34	13	7	7	66.4	188.7	34.2	55.3
10-3 10-9	6-9	55.5	35	14	7	7	67.4	278.8	33.5	59.2
11-1	6-10 7-0	58.4 61.4	36 37	13 14	9	7 7	77.5	139.6	36.8	65.2
11-5	7-1	64.4	38	15	9	7	77.8	172.0	36.1	69.3
11-9	7-2			-			78.2	222.0	35.3	73.3
12-3	7-3	67.5 70.5	39	16 15	9	7 7	78.7	309.5	34.4	77.1
12-7	7-5	73.7	41	16	11	7	90.8 90.5	165.2 200.0	38.4	83.4
12-11	7-6	77.0	42	17	11	7	90.4	251.7	36.5	87.4 91.3
13-1	8-2	83.0	43	18	13	6	88.88	143.6	42.0	93.6
13-1	8-4	86.8	44	21	11	6	81.7	300.8	35.8	93.7
13-11	8-5	90.3	45	18	15	6	100 4	132.0	46.0	103.3
14-0	8-7	94 2	46	21	13	6	90.3	215 1	39.4	104.5
13-11	9-5	101.5	47	23	14	5 5	86.2	159 3	42.8	103 9
14-3	9-7	105.7	48	24	14	5	87.2	176.3	42.0	107.0
14-8	9-8	109.9	49	24	15	5 5	90.9	166.2	44 0	112.3
14-11	9-10	114.2	50	25	15		91.8	183.0	43.2	115.5
15-4	10-0	118.6	51	25	16	5	95.5	173.0	45.3	120.8
15-7 16-1	10-2	123.1	52	26	16	5	96.4	189 6	44 4	123.9
16-4	10-6	132.3	53 54	26 27	17	5	100.2	179.7	46.6	129.2
16-9	10-8	136.9	55	27	18			196.1	45 7	132.3
17-0	10-10	141 8	56	28	18	5	105.0	186 3	47.9	137.7
17-3	11-0	146.7	57	29	18	5	106.5	202.5	46.9 45.9	140.8
17-9	11-2	151.6	58	29	19	5	110.4	208.9	48.2	149.3
18-0	11-4	156.7	59	30	19		111 1	227 3	47.2	152.3
18-5	11-6	161.7	60	30	20	5	115.2	215.2	49.6	157 8
18-8	11-8	167.0	61	31	20	5 5	115.8	233.3	48.5	160 7
19-2	11-9	172.2	62	31	21		1199	221 5	50.9	166.2
19-5	11-11	177.6	63	32	21	5	120 5	239.3	49 8	169.2
19-10	12-1	182.9	64	32	22	5	124 7	227 7	52.3	174 8
20-1 20-1	12-3	188.5	65 66	33 35	22	5	125.2	245.3 310.8	51 1	177.7
20-10	12-7		67	34	23				46 2	_
21-1	12-9	199 7	68	35	23	5	130.0	251 2 270 9	52 5 51 2	186.2
21-6	12-11	211.2	69	. 35	24	5	134 8	257 2	53 9	194 8
		2			~		1,54.0	231 6	20.3	1 .340

**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued



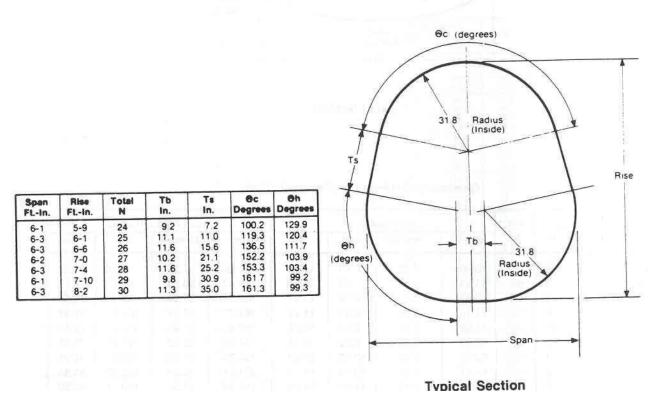
**Typical Section** 

### Geometric Data-Vehicular Underpass

Spa	an	Ri	se	Tot		Require	d N			nside Radiu	s (Inches)	
Ft	In.	Ft	ln.	N	Invert	Haunch	Side	Crown	Invert	Haunch	Side	Crown
12	1	11	0	47	10.00	4.32	7.69	12.99	135.95	37.95	88.00	67.95
12	10	11	2	49	11.04	4.44	7.50	14.10	148.53	38.53	86.78	74.53
13	0	12	0	51	10.97	4.27	8.79	13.91	160.54	37.54	98.19	72.54
13	8	12	4	53	11.98	4.36	8.67	14.96	167.77	37.77	102.62	76.77
14	0	12	11	55	11.99	4.39	9.62	14.98	182.90	37.90	110.65	76.90
14	6	13	5	57	13.07	4.61	9.26	16.18	174.88	38.88	124.73	78.88
14	8	14	1	59	13.00	4.42	10.58	15.99	192.96	37.96	130.01	78.96
15	5	14	5	61	14.04	4.59	10.33	17.11	201.54	38.54	135.39	83.54
15	6	15	2	63	13.97	4.45	11.61	16.92	211.59	37.59	149.14	81.59
16	2	15	6	65	14.99	4.50	11.52	17.97	216.85	37.85	154.40	85.89
16	6	16	0	67	14.07	4.73	12.10	19.29	272.34	39.34	153.89	89.34
16	8	16	4	68	15.01	4.49	12.49	19.03	246.17	38.17	160.82	89.17
17	3	17	1	70	15.04	5.71	12.20	19.13	214.64	47.64	171.19	90.64
18	5	16	11	72	16.09	5.87	11.95	20.27	249.37	48.37	155.02	100.37
19	0	17	3	74	17.02	5.60	12.36	21.06	262.29	47.29	153.14	105.2
19	7	17	7	76	17.07	5.79	13.06	21.24	296.21	48.21	154.46	108.2
20	5	17	9	78	18.08	5.78	13.05	22.27	317.39	48.39	149.94	115.3

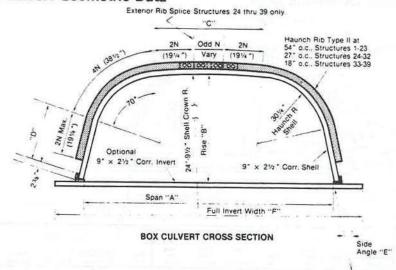
**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued

### Geometric Data-Pedestrian/Animal Underpass



**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued

### **Box Culvert Geometric Data**



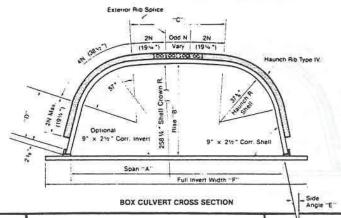
						SI	HELL						FULL IN	VERT	
Structure Number	Span "A" (Ftin.)	Rise "8" (FtIn.)	Ares (Sq. FL)	Crown (N)	ro" (N)	Side Angle "E" Deg. Min.	Total N	Plate Length (N)	Crown Plats Length (N)	Bolts/Ft.	Width "F" (N)	Suppleme Thick.	Width (N)	Weight/Ft.	Boits/Ft
1	8- 9	2- 6	18.4	5	.5	15-24	14	1@14	-	6.67	13	-	_	23.06	578
2	9- 2	3- 3	25.4	5	1.5	15-24	16	2@8	-	11.56	13	-	-	23.06	5.78
3	9- 7	4-1	32.6	5	2.5	15-24	18	2@ 9	-	12.00	14	-	-	24.44	6 00
4	10- 0	4-10	40.2	5	3.5	15-24	20	2@10	-	12.44	14	-	-	24.44	6.00
5	10- 6	5- 7	48.1	5	4.5	15-24	22	2@11	-	12.89	15	-	-	25.82	6.22
6	10-11	6- 4	56.4	5	5.5	15-24	24	2 @ 12	4	13.33	17	-	-	28.58	6.57
7	11- 4	7- 2	65.0	5	6.5	15-24	26	2@13	-	13.78	17	-	_	28.58	6.67
8	10- 2	2- 8	23.0	7	.5	13-33	16	2@8	_	12.89	15	_	_	25.82	6.22
9	10- 7	3- 5	31.1	7	1.5	13-33	18	2@ 9	-	13.33	15	_	_	25.82	6.22
10	10-11	4- 3	39.5	7	2.5	13-33	20	2@10	-	13.78	17	_	_	28.58	6.37
11	11- 4	5- 0	48.2	7	3.5	13-33	22	2@11	-	14,22	17	_	_	28.58	6.57
12	11-8	5- 9	57.2	7	4.5	13-33	24	2@12	-	14.67	17	_	_	28.58	6.57
13	12- 1	6- 7	66.4	7	5.5	13-33	26	2@13	_	15.11	17	_	-	28.58	6.37
14	12- 5	7- 4	75.0	7	6.5	13-33	28	2@14	-	15,56	17	_	_	28.58	6.37
15	11- 7	2-10	28.1	9	0.5	11-42	18	2@ 9	_	14,67	17	_	-	28.58	6.37
16	11-11	3- 7	37.4	9	1.5	11-42	20	2@10	-	15.11	17	_	_	28.58	6.37
17	12- 3	4- 5	46.9	9	2.5	11-42	22	2@11	_	15,56	17	-	_	28.58	6 67
18	12- 7	5- 2	56.6	9	3.5	11-42	24	2@12	-	16.00	19	_		32.02	7.11
19	12-11	6- 0	66.6	9	4.5	11-42	26	2@13	_	15.44	19	-	_	32.02	7.11
20	13- 3	6- 9	76.9	9	5.5	11-42	28	2@14	-	16.89	19	-	_	32.02	7.1
21	13- 0	3- 0	33.8	11	0.5	9-52	20	2@10	_	16.44	19	_	_	32.02	7.11
22	13- 4	3-10	44.2	11	1.5	9-52	22	2@11	-	16.89	19		_	32.02	7.11
23	13- 7	4- 7	54.8	11	2.5	9-52	24	2@12	-	17.33	19	-	_	32.02	7.11
24	13-10	5- 5	65.6	11	3.5	9-52	26	2@13	_	23.11	19	_	_	32.02	7,11
25	14- 1	6- 2	76.6	11	4.5	9-52	28	2@14	-	23.56	20	_	-	33.34	12.14
26	14- 5	3- 3	40.0	13	0.5	8-1	22	2@11	-	22.67	20	_		33.34	12.14
27	14- 8	4- 1	51.5	13-	1.5	8-1	24	2@8	8	25.56	21	100	2	40.23	12.57
28	14-10	4-10	63.2	13	2.5	8-1	26	2@ 9	8	26.44	21	.100	2	40.23	12.37
29	15- 1	5- 8	75.1	13	3.5	8-1	28	2 @ 10	8	26.89	21	.100	2	40.23	12.37
30	15- 4	6- 5	87.2	13	4.5	8-1	30	2@11	8	27.33	21	.100	2	40.23	12.37
31	15- 6	7- 3	99.4	13	5.5	8-1	32	2 @ 12	8	27.78	22	,100	2	41.61	12.49
32	15- 9	8- 0	111.8	13	6.5	8-1	34	2@13	8	28.22	22	.100	2	41.61	12.39
33	15-10	3- 5	46,8	15	0.5	6-10	24	2@8	8	32.22	22	.100	2	41.61	12.39
34	16- 0	4- 3	59.5	15	1.5	6-10	26	20 9	8	33.56	22	.100	2	41.61	12.49
35	16- 2	5- 1	72.3	15	2.5	6-10	28	2 @ 10	8	34.89	23	.100	2	42.99	13.11
36	16- 4	5-11	85.2	15	3.5	6-10	30	2 @ 11	8	35.33	23	,100	3	45.75	13,11
37	16- 6	6- 8	98.3	15	4.5	6-10	32	2 @ 12	8	35.78	23	.100	3	45.75	13.11
38	16- 8	7- 6	111.5	15	5.5	6-10	34	2 @ 13	8	36.22	23	.100	3	45.75	13.11
39	16-10	8- 3	124.8	15	6.5	6-10	36	2 @ 14	8	36.67	24	.100	3	47.13	13.13

#### NOTES:

- 1) "N" equals 9.62".
- 2) All crowns of shells have Type IV ribs outside at 18" on centers.
- 3) Weights per foot listed do not include bolt weight.
- 4) Weight per foot of full invert includes 3½ x 3x ½ connecting angle and scalloped closure plate for each side. Inverts for 20N and greater are two-piece.
- Weight per foot of footing pad includes a 3½ x 3x ¼-in, connecting angle for each side. Optional wale beam not included.
- 6) Full invert plates are .100 thick. When reactions to invert require additional thickness supplemental plates of thickness and width listed are furnished to bolt between full invert and side connecting angle.
- 7) Width of footing pad is for each side.
- For structures using short footing pads with leg length "D" equal to
   3.5 N or more, either wale beam stiffeners should be used to avoid

**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued

#### **Box Culvert Geometric Data**

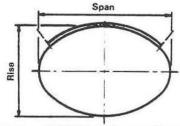


							SH	ELL				\ FUL	L INVERT		
Hructurs Humber	Span "A" (Ft-In.)	Rise "8" (Ftin.)	Area (Sq. Ft.)	Crown Width "C" (N)	Leg Length "D" (N)	Side Angle "E" Deg. Min.	Total N	Haunch Plate Length (N)	Crown Plats Length (N)	Bolts Per Foot	Width "F" (N)	Supplementa Thickness	Width (N)	Weight Per Foot	Boits Per Foo
40	17. 9	3-10	54.4	17	.5	14-54	26	8	10	33.56	25	.100	3	48.51	13,56
41	18- 2	4. 7	68.3	17	1.5	14-54	28	9	10	34.89	25	.100	3	48.51	13.56
42	18- 7	5- 4	82.5	17	2.5	14-54	30	10	10	36.22	26	.100	3	49.88	13.78
43	19- 0	6- 1	97.1	17	3.5	14-54	32	7.7	10	36,67	27	.100	3	51.26	14.00
44	19- 5	6-11	111.9	17	4.5	14-54	34	12	10	37.11	27	.100	3	51.26	14.00
45	19-10	7-8	127.1	17	5.5	14-54	36	13	10	37.56	28	.100	3	52.64	14.22
45	20- 3	8-5	142.6	17	6.5	14-54	38	14	10	38.00	28	.100	3	52.64	14,22
47	19- 1	4. 2	63.3	19	.5	12-47	28	В	12	34.89	27	.100	3	51.26	14.00
48	19- 5	4-11	78.3	19	1.5	12-47	30	9	12	36.22	27	.100	3	51.26	14.00
49	19 9	5-8	93.6	19	2.5	12-47	32	10	12	37.56	27	.100	3	51.26	14.00
50	20- 1	6- 6	109.2	19	3.5	12-47	34	11	12	38.00 54.44	28 29	.100	3	52,64	14.22
51	20- 6	7- 3	125.0	19	4.5 5.5	12-47	38	13	12	54.89	29	.100	3	54.02	14,44
52 53	20-10	8- 1 8-10	141.2	19	5.5	12-47	40	14	12	55.33	30	.150	3	59.54	14.67
(T)(1)-1	10.7.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1.1		73.1	21	.5	10-40	30	8	14	49.56	29	.150	3	58.16	14.44
54	20- 4	4- 6 5- 3	89.2	21	1.5	10-40	32	9	14	52.22	29	.125	3	56.09	14,44
55 56		6 1	105.5	21	2.5	10-40	34	10	14	54.89	29	.100	3	54.02	14.44
57	20-11	6-10	122.1	21	3.5	10-40	36	11	14	55.33	30	.150	3	59.54	14,67
58	21- 6	7- 8	139.0	21	4.5	10-40	38	12	14	55.78	30	125	3	57.47	14.67
59	21-10	8- 5	156.0	21	5.5	10-40	40	13	14	56.22	31	175	3	62.99	14.89
60	22- 1	9- 3	173.3	21	6.5	10-40	42	14	14	56.67	31	150	3	60.92	14.89
61	21- 7	4-11	83.8	23	.5	8-32	32	9	14	50.89	30	.125	3	57.47	14.67
62	21-10	5- 8	101.0	23	1.5	8-32	34	10	14	53.56	31	.175	3	62.99	14.89
63	22- 1	6- 6	118.4	23	2.5	8-32	36	11	14	56.22	31	150	3	60.92	14.89
64	22- 3	7- 3	135.9	23	3.5	8-32	38	12	14	56.67	31	.150	4	65.05	14.89
65	22- 6	8- 1	153.7	23	4.5	8-32	40	13	14	57.11	32	.200	4	71.95	15.11
66	22- 9	8-10	171.6	23	5.5	8-32	42	14	14	57.56	32	.175	4	69.19	15.11
67	23- 0	9-8	189.8	23	6.5	8-32	44	15	14	58.00	3.2	.150	4	66.43	15.11
68	22- 9	5- 4	95.5	25	.5	6-25	34	10	14	52.22	32	.175	4	69.19	15.11
69	23- 0	6- 1	113.7	25	1.5	6-25	36	11	14	54.89	32	.150	4	66.43	15.11
70	23- 2	6-11	132.1	25	2.5	6-25	38	12	14	57.56	33	225	4	76.09	15.33
71	23- 4	7- B	150.6	25	3.5	6-25	40	13	14	58.00	33	.200	4	73.33	15.33
72	23- 6	8-6	169.3	25	4.5	6-25	42	14	14	58.44	33	200	4	73.33	15.33
73	23- 8	9-3	188.1	25	5.5	6-25	44	15	14	58.89	33	.175	4	70.57	15.33
74	23-10	10- 1	207.0	25	6.5	6-25	46	16	14	59.33	34	.250	4	80.22	15.56
75	24- 0	5- 9	108.2	27	.5	4-18	36	10	16	53.56	34	.225	4	77.46	15.56
76	24 1	6- 6	127.5	27	1.5	4-18	38	11	16	56.22	34	.225	4	77.46	15.56
77	24 3	7- 4	145.8	27	2,5	4-18	40	12	16	58.89	34	.200	4	74.71	15.56
78	24 4	8- 2	166.2	27	3.5	4-18	42	13	16	59.33	34	.200	4	74.71	15,56
79	24 5	8-11	185.7	27	4.5	4-18	44	14	16	59.78	34	.200	4	74.71	15,56
80	24 7	9- 9	205.3	27	5.5	4-18	46	15	16	60.22	35	.300	4	87.12	15.78
81	24- 8	10- 6	225.0	27	6.5	4-18	48	16	16	60.67	35	.250	4	81.60	15.78
82	25- 2	6- 2	122.0	29	.5	2-11	38	11	16	54.89	35	.200	4	76.09	15.78
83	25- 2	7- 0	142.2	29	1.5	2-11	40	12	16	57.56	35	.200	4	76.09	15.78
84	25- 3	7- 9	162.4	29	2.5	2-11	42	13	16	60.22	36	.300	4	88.50	16,00
85	25- 4	8- 7	182.6	29	3.5	2-11	44	14	16	60.67	36	.300	4	88.50	16.00
86	25 4	9-5	202.9	29	4.5	2-11	46	15	16	61.11	36	.300	4	88,50	16,00
87	25- 5	10- 2	223.3	29	5.5	2-11	48	16	16	61,56	36	.300	4	88.50	16.00

NOTES:

- 1) "N" = 9.62"
- All shells have Type IV ribs outside only. Both haunch and crown ribs are 18" on centers for structures 40 through 50 and 9" on centers for structures 51 through 87.
- 3) Weights per foot listed do not include bolt weight.
- 4) Weight per foot of full invert includes 3½ x3x.½ connecting angle and scalloped closure plate for each side. Inverts for 20 N width and greater are two piece.
- 5) Full invert plates are 100° thick. When reactions to invert require
- additional thickness, supplemental plates of thickness and width listed are furnished to boit between full invert and side connecting angles. When thickness listed is greater than a .250° supplemental plates will be two pieces equalling the composite thickness required.
- Weight per foot of footing pads includes 3½ x3x ¼ connecting angle for each side. Optional wale beam weight is not included.
- Width of footing pads is for each side. When thickness listed is greater than .250" the footing pads will be two pieces equalling the composite thickness required.

**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued

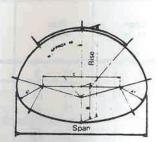


				Required N		Inside	Radius
Span Ftin	Rise Ft-in.	Area ft²	Crown or invert	Haunch	Total	Crown & Invert in.	Haunch in.
19 4	12 9	191	22	10	64	150.3	53.9
20 1	13 0	202	23	10	66	157.2	53.9
20 2	11 10	183	24	8	64	164.1	42.8
20 10	12 2	193	25	8	66	171.0	42.8
21 0	15 1	248	23	13	72	157.2	70.4
21 11	13 1	220	26	9	70	177.9	48.4
22 6	15 8	274	25	13	76	171.0	70.4
23 0	14 1	249	27	10	74	184.8	53.9
23 3	4 16 11 320		26	13	78	177.9	70.4
24 4	6 14 7 274		27	14	82	184.8	75.9
24 6	3 14 11 287		29	10	78	198.6	53.9
25 3	6 16 9 330		30	10	80	205.4	53.9
25 6 26 1 26 3 27 0	16 9 18 2 15 10 16 2	330 369 320 334	29 29 31 32	13 15 11	84 88 84 86	198.6 198.6 212.3 219.2	70.4 81.4 59.4 59.4
27 2	19 1	405	30	16	92	205.4	86.9
27 11	19 5	421	31	16	94	212.3	86.9
28 1	17 1	369	33	12	90	226.1	64.9
28 10	17 4	384	34	12	92	233.0	64.9
29 5	19 11	455	33	16	98	226.1	86.9
30 2	20 2	472	34	16	100	233.0	86.9
30 4	17 11	415	36	12	96	246.8	64.9
31 2	21 2	513	35	17	104	239.9	92.5
31 4	18 11	454	37	13	100	253.7	70.4
32 1	19 2	471	38	13	102	260.6	70.4
32 3	22 2	555	36	18	108	246.8	98.0
33 0	22 5	574	37	18	110	253.7	98.0
33 2	20 1	513	39	14	106	267.5	75.9
34 1	23 4	619	38	19	114	260.6	103.5
34 8	20 8	548	41	14	110	281.2	75.9
35 0	21 4	574	41	15	112	281.2	81.4
35 2	24 4	666	39	20	118	267.5	109.0
35 10	25 9	719	39	22	122	267.5	120.0
36 1	22 4	620	42	16	116	288.1	86.9
36 11	25 7	736	41	21	124	281.2	114.5
37 2	22 2	632	44	15	118	301.9	81.4
38 0	26 7	786	42	22	128	288.1	120.0
38 8	28 0	844	42	24	132	288.1	131.0
40 1	29 8	928	43	26	138	295.0	142.1

SOURCE: ALUMINUM ASSOCIATION

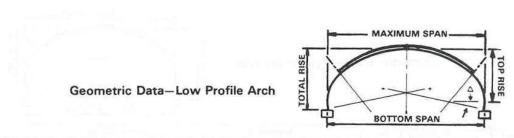
**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued

Geometric Data-Pipe Arch



s	pan	R	180	Area		Requi	red N		Inside	Radius		
	t-in		in.	M2	Total	Crown	Invert	Haunch	Crown in.	Invert in.	8	С
20	1	13	11	216	68	34	20	7	122.7	224.2	62.9	146.7
20	7	14	3	229	70	36	20	7	124.9	256.4	61.4	152.8
21	5	14	7	241	72	36	22	7	131.7	237.3	65.4	163.4
21	11	14	11	254	74	38	22	7	133.7	268.8	63.8	169.4
22	8	15	3	267	76	39	23	7	138.2	274.9	65.0	177.8
23	4	15	7	281	78	40	24	7	142.7	281.0	66.3	186.1
24	3	15	10	295	80	40	26	7	150.0	262.8	70.8	196.8
24	9	16	3	309	82	42	26	7	151.7	293.0	68.9	202.9
25	5	16	7	324	84	43	27	7	156.2	299.0	70.2	211.3
26	4	16	10	339	86	43	29	7	163.9	281.3	75.0	222.1
27	0	17	2	354	88	44	30	7	168.6	287.4	76.4	230.5
27	9	17	6	369	90	45	31	7	173.3	293.5	77.9	238.9
28	5	17	10	385	92	46	32	7	178.0	299.6	79.3	247.3
29	4	18	2	401	94	46	34	7	186.6	286.7	84.6	257.9
29	10	18	6	418	96	48	34	7	187.5	311.6	82.3	264.2
30	4	18	10	435	98	50	34	7	188.6	340.1	80.0	270.2

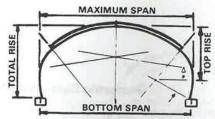
**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued



M	ax.	T	otal	Area	Bot	tom	T	ор		Required N		Inside	Radius		7
	oan -in.		t-in	ft²		an -in.		ise -in.	Crown	Side	Total	Crown In.	Side In.	Deg	. Min
20	1	7	6	120	19	10	6	6	23	6	35	157.2	54.0	12	19
19	5	6	9	105	19	2	5	10	23	5	33	157 2	43.0	15	22
21	6	7	9	133	21	4	6	9	25	6	37	1710	54.0	12	19
22	3	7	11	140	22	1	6	11	26	6	38	1779	54.0	12	19
23	0	8	0	147	22	10	7	1	27	6	39	184.8	54.0	1 12	19
23	9	8	2	154	23	6	7	2	28	6	40	191.7	54.0	12	19
24	6	8	3	161	24	3	7	4	29	6	41	196.6	54 0	12	19
25	3	. 8	5	168	25	0	7	5	30	6	42	205 4	54.0	12	19
26	C	8	7	175	25	9	7	7	31	6	43	212.3	54.0	12	19
27	3	10	0	217	27	1	9	0	31	8	47	212.3	76.0	8	51
28	1	9	6	212	27	11	8	7	33	7	47	226.1	65.0	10	17
28	9	10	3	234	28	7	9	3	33	8	49	226.1	75.0	8	52
28	10	9	8	220	28	8	8	8	34	7	48	233.0	65.0	10	17
30	4	. 9	11	237	30	2	9	0	36	7	50	246.8	65.0	10	17
31	0	j 10	8	261	30	10	9	8	36	8	52	246.8	76.0	8	52
31	7	12	1	309	31	2	10	4	36	10	56	246.8	87 0	14	0
31	1	10	1	246	30	10	9	1	37	7	51	253.7	65.0	10	17
32	4	12	3	319	31	11	10	6	37	10	57	253.7	87.0	14	0
31	9	10	2	255	31	7	9	3	38	7	52	260.6	65.0	10	17
33	1	1 12	5	330	32	8	10	8	38	10	58	260.6	87.0	14	0
33	2	111	0	289	33	0	10	-1	39	8	55	267.5	76.0	8	52
34	6	13	3	367	34	1	11	6	39	11	61	267 5	98.0	12	26
34	8	11	4	308	34	6	10	4	41	8	57	281.2	76.0	8	52
37	11	15	7	478	37	8	13	10	41	14	69	281.2	131 0	9	23
35	5	111	5	318	35	3	10	6	42	8	58	288.1	76.0	8	52
38	8	15	9	491	38	4	14	0	42	14	70	288.1	131.0	9	23

**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued

Geometric Data-High Profile Arch

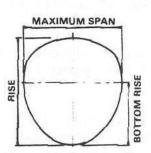


M	ax		tal	Area	Bot	tom	Top		Requ	ired N			Inside Radi	us	1 4	1
Sp Ft.		1000	ise -in.	ft²	Sp Ft-	en in.	Rise Ft-ir		n Haunch	Side	Total	Crown in.	Haunch in.	Side in.	Deg.	Min.
20	1	9	1	152	19	6	6	6 23	5	3	39	157.2	54 0	157.2	11	40
20	9	12	1	214	18	10	7	3 23	6	6	47	157 2	65 0	157 2	22	8
21	6	11	8	215	19	10	6	9 25	5	6	47	171 0	54 0	171 0	20	20
22	10	14	6	284	19	10	8	6 25	7	8	55	1710	760	171 0	26	48
22	3	11	9	224	20	7		11 26	5	6	48	177.9	540	177.9	19	33
22	11	14	0	275	20	1	7	7 26	6	8	54	177 9	65 0	177.9	25	44
23	0	11	11	234	21	5	7	1 27	5	6	49	184 8	540	184 8	18	49
24	4	14	10	309	21	7	8	5 27	7	8	57	184 8	760	184 8	24	50
23	9	12	.1	244	22	2	7	2 28	1 5	6	50	191.7	540	191 7	18	8
24	6	13	8	288	21	11	7	4 29	5	8	55	198 6	54 0	198 6	23	2
25	10	15	1	334	23	3	8	9 29	7	8	59	198 6	76.0	198 6	23	6
25	3	13		283	23	3	7	5 30	5	7	54	205 4	540	205 4	19	35
26	6	15	3	347	24	0	8	0 30	7	8	60	205 4	76.0	205 4		_
26	0	13	3	294	24	1	7	7 31	5	7	55	2123	540	212 3	22	19
27	3	15	5	360	24	10	9	0 31	7	8	61	2123	76.0	212 3	18	57
27	5	13	6	317	25	8		0 33	5	7	57	226 1	540	226 1	21	36
29	5	16	5	412	27	1	10	0 33	8	8		226 1			17	48
28	2	14	5	348	25	11	8	0 34	5	8	65	233 0	870	226 1	20	18
30	2	18	0	466	26	8	10	2 34	8	10	60		54 0	233 0	19	37
30	4	15	5	399	28	2	9	0 36	6	8	70 64	233 0	88 0	233 0	23	51
31	7	18	4	497	28	5	10	4 36	8				65 0	246 8	18	34
31	1	15	7	412	29	o	9	1 36		10	72	246 8	870	246 8	23	3
31	8	17	9	483	28	7		10 37	6 7	8	65	253 7	65 0	253.7	18	3
32	4	19	11	554	27	11	10	6 37		10	71	253 7	76.0	253 7	22	25
31	9	_	-		_				8	12	77	253 7	87.0	253 7	26	45
33	1	17	2	469	28	9	9	3 38	6	10	70	260 6	650	260 6	21	47
32	6	17	1	571	28	9	10	8 38	8	12	78	260 6	87.0	260 6	26	3
33	10	20	4	484	29	6	9	4 39	6	10	71	267 5	65 0	267.5	21	14
	_		3	588	29	7	10	9 39	8	12	79	267.5	870	267 5	25	23
34	0	17	8	514	31	2	9	8 41	6	10	73	281 2	65.0	281 2	20	11
	8	19	10	591	30	7	10	4 41	7	12	79	281 2	760	281.2	24	7
35	4	21	3	645	30	7	11	0 41	8	13	83	281 2	870	281 2	26	6
_	3	23	4	747	32	7	13	2 41	11	13	89	281 2	1200	281 2	26	8
34	9	17	9	529	31	11	9	9 42	6	10	74	288 1	65 0	288 1	19	42
35	5	20	0	608	31	5	10	6 42	7	12	80	288 1	760	288 1	23	33
36	1	21	5	663	31	5	11	2 42	8	13	84	288 1	87.0	288 1	25	28
38	0	23	6	767	33	5	13	3 42	11	13	90	288 1	120 0	288 1	25	31

See "Notes" Table 5-20A or 5-20B for rib spacing when required.

**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued

### Geometric Data-Pear Shape



Ma	ex.	-		Ri	ise			F	beriupes	N		Leplain.	Inside	Radius	
	สก	1	ise -in.	1	in.	Area ft²	Тор	Gorner	Side	Bottom	Total	Bottom in.	Side in.	Corner in.	Top in.
23 24 25 24	7 0 4 10	25 25 25 27	6 10 11 7	14 15 15 16	10 1 10 9	477 497 518 545	25 22 27 27	5 7 7 5	24 22 20 25	15 20 20 18	98 100 102 105	108.31 119.07 124.23 110.90	198.07 208.07 218.24 236.21	74 07 84 07 84 24 69 21	175.07 194.07 191.24 191.21
28 26 28 28	10 8 0 7	27 28 27 30	3 3 10 7	19 18 16 19	8 0 9 7	590 594 624 690	32 28 27 32	7 5 8 7	27 30 22 24	8 12 25 24	110 110 112 118	79.61 95.45 146.38 133.13	257.96 241.24 227.72 288.45	68.96 57.24 86.72 84.45	252.96 251.24 244.72 218.45
30 30	0	29 31	7 2	20 19	0	699 739	32 34	8 7	23 24	25 26	119	142.41 144.43	288.26 -288.58	79 26 84 58	262.26 231.58

**Figure 12.4.45** Standard Sizes for Aluminum Culvert (Source: Aluminum Association), continued

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